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STRUCTURAL – ADDENDUM 1

July 22, 2026 **Project Number:** 21032

Project Name: The Sterling Hall School

The following are changes to the tender documents and are effective immediately. This addendum forms part of the contract documents issued previously. All changes are clouded in drawings.

1 PART 1 - SPECIFICATIONS

1.1 NONE

2 PART 2 - DRAWINGS

2.1 DRAWING S000

2.1.1 Deleted drawing sheets

2.2 DRAWING S100, S101, S102, S108

2.2.1 Notes is revised as clouded

2.2.2 Deleted drawing sheets and rearranged/modified the typical details in the drawing sheet indicated

2.3 DRAWING S210

2.3.1 Added notes as clouded

2.3.2 Added frost slabs at entrances

2.3.3 Revised wood deck framing

2.4 DRAWING S230

2.4.1 Revised concrete beam and reinforcement.

2.4.2 Revised structural steel framing

2.5 DRAWING S240

2.5.1 Added shear stud rails at columns

2.5.2 Added masonry knee wall for brick veneer support

2.5.3 Revised structural steel framing

2.6 DRAWING S250

2.6.1 Revised structural steel framing

2.6.2 Added connection plate and bearing plate

2.7 DRAWING S310, S311, S400

2.7.1 Revised connection plate schedule

2.7.2 Added shear steel stud rail schedule

2.7.3 Revised concrete shear wall typical detail

2.8 DRAWING S401, S402, S403

2.8.1 Revised and added elevations

2.8.2 Revised reinforcement

2.9 DRAWING S410, S411

2.9.1 Added/revised connection plate and structural steel framing

2.10 DRAWING S500, S501, S502, S503, S510, S511, S521, S530, S531, S540, S541

2.10.1 Revised and/or added Section 1/S500, 4/S500, 5/S501, 6/S501, 2/S502, 2/S503, 3/S503, 8/S503, 10/S503, 5/S510, 3/S511, 4/S511, 5/S511, 6/S511, 7/S511, 2/S521, 4/S521, 3/S530, 4/S530, 5/S530, 7/S530, 11/S530, 12/S530, 1/S531, 8/S531, 9/S531, 10/S531, 12/S540, 10/S541, 11/S541, 12/S541

2.11 DRAWING S600, S601, S602, S603

2.11.1 Revised reinforcement and elevations for the stairs

2.12.1 Added elevations to show reinforcement details

* * *

END OF STRUCTURAL ADDENDUM NO. 1

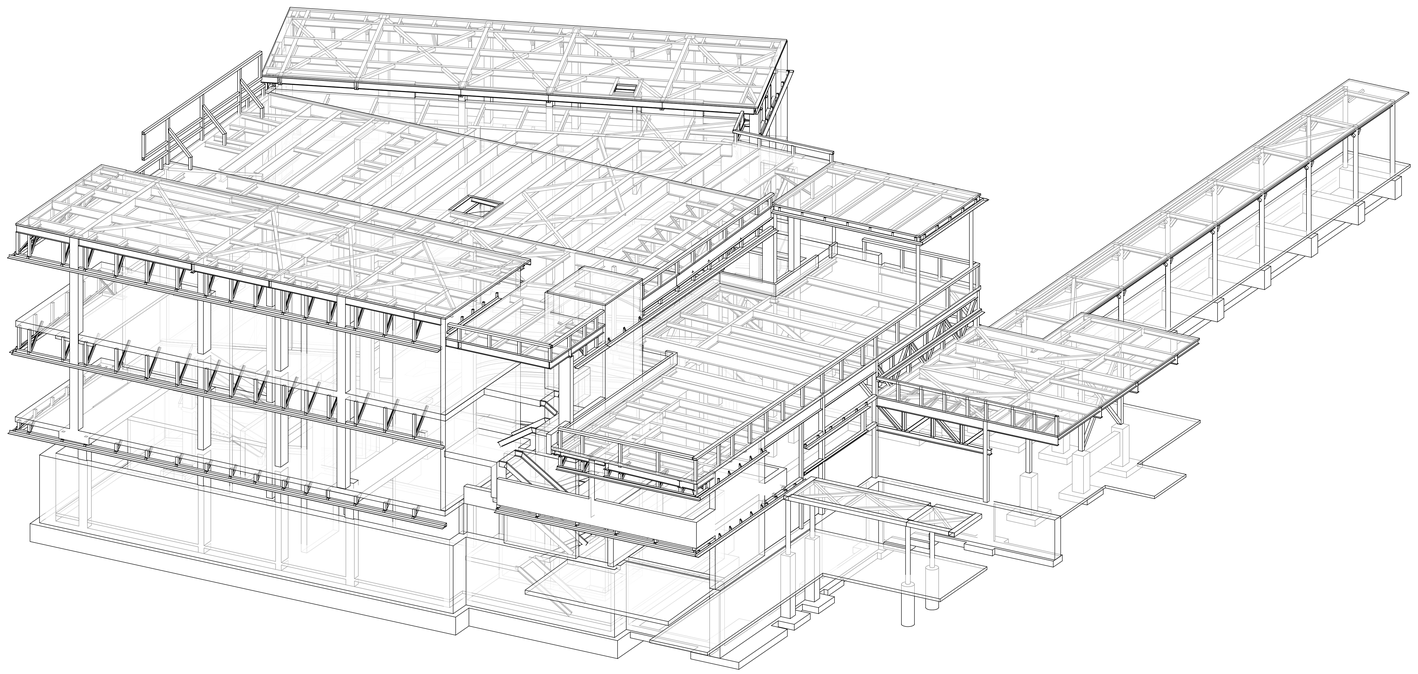
ATTACHMENTS: DRAWINGS S000, S100, S101, S102, S108, S210, S230, S240, S250, S310, S311, S400, S401, S402, S403, S410, S411, S500, S501, S502, S503, S510, S511, S521, S530, S531, S540, S541, S600, S601, S602, AND S603

SHEET LIST

SHEET NUMBER	SHEET NAME
S000	COVER PAGE
S100	GENERAL NOTES
S101	GENERAL NOTES
S102	TYPICAL DETAILS
S103	TYPICAL DETAILS
S104	TYPICAL DETAILS
S105	TYPICAL DETAILS
S106	TYPICAL DETAILS
S107	TYPICAL DETAILS
S108	TYPICAL DETAILS
S109	TYPICAL DETAILS
S110	TYPICAL DETAILS
S120	MEZZANINE AND SECOND FLOOR ROOF UPLIFT DIAGRAM
S121	ROOF UPLIFT DIAGRAM
S200	FOUNDATION AND LOWER LEVEL FRAMING PLAN
S210	GROUND FLOOR FRAMING PLAN
S220	UPPER GROUND FLOOR FRAMING PLAN
S230	MEZZANINE LEVEL FRAMING PLAN
S240	SECOND FLOOR FRAMING PLAN
S250	LOW ROOF FRAMING PLAN
S260	HIGH ROOF FRAMING PLAN
S300	COLUMN SCHEDULE
S301	COLUMN SCHEDULE
S310	SCHEDULES
S311	SCHEDULES
S400	SHEAR WALL KEY PLAN AND SCHEDULE
S401	SHEAR WALL ELEVATIONS
S402	SHEAR WALL ELEVATIONS
S403	SHEAR WALL ELEVATIONS
S406	FRAMING ELEVATIONS
S410	BUILDING ELEVATIONS
S411	BUILDING ELEVATIONS
S412	BUILDING ELEVATIONS
S500	FOUNDATION SECTIONS
S501	FOUNDATION SECTIONS
S502	FOUNDATION SECTIONS
S503	FOUNDATION SECTIONS
S510	GROUND FLOOR SECTIONS
S511	GROUND FLOOR SECTIONS
S520	MEZZANINE FLOOR SECTIONS
S521	MEZZANINE FLOOR SECTIONS
S530	SECOND FLOOR SECTIONS
S531	SECOND FLOOR SECTIONS
S540	ROOF FLOOR SECTIONS
S541	ROOF FLOOR SECTIONS
S600	STAIR PLANS AND SECTIONS
S601	STAIR PLANS AND SECTIONS
S602	STAIR PLANS AND SECTIONS
S603	STAIR PLANS AND SECTIONS

6

6



3D MODEL DISCLAIMER

ELEMENTS REPRESENTED IN 3D MODEL VIEW ARE SHOWN FOR INFORMATION PURPOSES ONLY AND MAY NOT REPRESENT DESIGN INTENT. THIS VIEW IS DIAGRAMATIC ONLY, AND NOT FOR CONSTRUCTION

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER
6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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COVER PAGE

scale:	1 : 1
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S000
	Revision: 6

A0 FILENAME: Autodesk Docs://2102 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-22 3:08:41 PM

GENERAL NOTES — GN-001
January 7, 2022

1. PROVIDE ALL MATERIAL AND LABOUR REQUIRED FOR THE COMPLETION OF THE WORK. BREAKDOWN OF WORK BY TRADE IS FOR GUIDANCE ONLY AND IS NOT NECESSARILY COMPLETE.
2. THIS IS A METRIC PROJECT. ALL LENGTHS ARE IN MILLIMETERS, UNLESS OTHERWISE NOTED.
3. DO NOT SCALE THESE DRAWINGS.
4. COMPLY WITH THE REQUIREMENTS OF THE ONTARIO BUILDING CODE AND THE OCCUPATIONAL HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS.
5. REVIEW STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE DRAWINGS PREPARED BY ALL CONSULTANTS PRIOR TO CONSTRUCTION. REPORT DISCREPANCIES BEFORE PROCEEDING WITH THE WORK.
6. FOLLOW TYPICAL DETAILS (I.E. SD-001) SHOWN ON DRAWING(S) S103-S113, UNLESS OTHERWISE NOTED ON DRAWINGS. TYPICAL DETAILS SHOWN ON DRAWINGS INTENT RATHER THAN ACTUAL CONDITIONS FOR THIS PROJECT. DETAILS NOTED IN TYPICAL DETAILS APPLY TO SIMILAR CONDITIONS THROUGHOUT THE PROJECT UNLESS OTHERWISE NOTED.
7. ALL SECTIONS, DETAILS, AND STATEMENTS NOTED AS "TYPICAL" APPLY TO LIKE/SIMILAR CONDITIONS IN THE STRUCTURE.
8. COORDINATE ALL OPENINGS, SLEEVES AND EMBEDDED ITEMS IN STRUCTURE WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND LANDSCAPE DRAWINGS. REPORT ANY ITEMS THAT ARE IN CONFLICT.
9. DO NOT CUT OR DRILL ANY OPENINGS IN STRUCTURAL MEMBERS WITHOUT WRITTEN PERMISSION FROM THE STRUCTURAL CONSULTANT UNLESS SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS.
10. USE OF THESE DRAWINGS IS LIMITED TO THAT IDENTIFIED IN THE REVISIONS COLUMN. DO NOT CONSTRUCT FROM THESE DRAWINGS UNLESS MARKED "ISSUED FOR CONSTRUCTION".
11. DO NOT USE INFORMATION ON THESE DRAWINGS FOR ANY OTHER PROJECT OR WORKS.

TEMPORARY WORKS — GN-002
December 22, 2021

1. DRAWINGS SHOW COMPLETED STRUCTURE ONLY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE SCOPE, DESIGN AND ERECTION OF ALL TEMPORARY WORKS THAT ARE NECESSARY TO COMPLETE THE WORK. FULL DESIGN LOADS MAY ONLY BE APPLIED AFTER THE CONCRETE REACHES ITS DESIGN STRENGTH.
2. CONSTRUCTION LOADS ON PERMANENT STRUCTURE SHALL NOT EXCEED DESIGN LOADS INDICATED IN THE CONTRACT DOCUMENTS.
3. DESIGN AND FIELD REVIEW OF ALL TEMPORARY WORKS TO BE CARRIED OUT BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED AND INSURED IN THE PROVINCE OF ONTARIO.
4. ANCHOR RODS AND OTHER EMBEDDED ITEMS HAVE BEEN DESIGNED FOR LOADS OF THE COMPLETED STRUCTURE ONLY AND ARE NOT TO BE USED OR RELIED UPON FOR TEMPORARY SUPPORT OR STABILITY DURING ERECTION UNLESS REVIEWED AND APPROVED BY THE CONTRACTOR'S ENGINEER RESPONSIBLE FOR THE ERECTION PROCEDURES.
5. MAKE GOOD ALL EXISTING WORK DISTURBED BY TEMPORARY CONSTRUCTION OPERATIONS.
6. EXTENT OF ALL TEMPORARY SHORING WHICH MAY BE REQUIRED IS NOT SHOWN ON STRUCTURAL DRAWINGS, CONTRACTOR TO DETERMINE. REFER TO SPECIFICATIONS FOR TEMPORARY SHORING REQUIREMENTS.

SUBMITTALS AND SHOP DRAWINGS — GN-003
March 8, 2022

1. SUBMIT FOR REVIEW ALL SUBMITTALS AND SHOP DRAWINGS REQUIRED BY THE SPECIFICATIONS, INCLUDING BUT NOT LIMITED TO THE FOLLOWING:
   - A. CONCRETE MIX DESIGNS
   - B. PROPOSED CONSTRUCTION AND CONTROL JOINT LOCATIONS IN CONCRETE ELEMENTS
   - C. CONCRETE AND MASONRY REINFORCING STEEL
   - D. STRUCTURAL STEEL (ERECTION DRAWINGS AND DETAILS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
   - E. STEEL JOISTS (ERECTION DRAWINGS, DETAILS AND CALCULATIONS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
   - F. STEEL DECK (SEALED AND SIGNED BY A PROFESSIONAL ENGINEER)
2. REVIEW OF SHOP DRAWINGS IS ON A RATIONAL SAMPLING BASIS, FOR GENERAL CONFORMITY WITH THE INTENT OF THE STRUCTURAL CONTRACT DOCUMENTS. IT IS NOT A DETAILED CHECK AND MUST NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR OF RESPONSIBILITY FOR MAKING THE WORK ACCURATE AND IN CONFORMITY WITH THE CONTRACT DOCUMENTS.
3. REVIEW OF SHOP DRAWINGS DOES NOT IMPLY ANY CHANGE IN ANY OTHER CONSULTANTS' OR PROFESSIONALS' RESPONSIBILITIES RELATED TO DESIGN OF SPECIFIC ITEMS AS OUTLINED BY THE SPECIFICATIONS.
4. REVIEW OF A SPECIFIC COMPONENT ON NON-STRUCTURAL SHOP DRAWINGS (COMPONENTS ENGINEERED BY OTHERS) DOES NOT INCLUDE REVIEW OF THE ASSEMBLY OF WHICH THE ITEM MAY BE A COMPONENT.
5. IF REQUIRED, CAD FILES OF THE FULL SET OF STRUCTURAL DRAWINGS ARE AVAILABLE "AS-IS", AT A COST OF $100.00 PER SHEET FOR USE IN THE PREPARATION OF SHOP DRAWINGS, PROVIDED THAT THE OWNER AND THE OWNER'S CONSULTANTS ARE NOT HELD RESPONSIBLE FOR ANY ERRORS OR OMISSIONS ON THE DRAWINGS. THESE CAD DRAWINGS ARE NOT TO BE SCALED.
6. ALLOW A MINIMUM OF 10 WORKING DAYS FOR REVIEW OF EACH SUBMISSION OF SHOP DRAWINGS IN THE STRUCTURAL OFFICE. ALLOW MORE TIME WHEN LARGE QUANTITIES OF SHOP DRAWINGS ARE SUBMITTED ALONG WITH A PRIORITY LIST OF SUBMITTALS. SUBMIT IN ORDER WITH THE INTENDED SEQUENCE OF CONSTRUCTION.
7. MAKE CORRECTIONS REQUIRED BY PREVIOUS REVIEW BEFORE RESUBMITTING DRAWINGS. CLEARLY INDICATE ALL CHANGES AND ADDITIONS TO PREVIOUS SUBMISSION. DO NOT ADD NEW DETAILS TO DRAWINGS WHICH HAVE BEEN STAMPED AS REVIEWED OR NOTED.
8. AFTER REVIEW, THE DRAWINGS WILL BE STAMPED AND RETURNED TO SHOW ONE OF THE FOLLOWING:

REVIEWED	RELEASED FOR FABRICATION
REVIEWED AS NOTED	RELEASED FOR FABRICATION AFTER REVISIONS NOTED ARE MADE
REVISE AND RESUBMIT	CORRECT AND RESUBMIT FOR REVIEW PRIOR TO FABRICATION.
NOT REVIEWED	SHOWS WORK WHICH IS NOT WITHIN THE SCOPE OF STRUCTURAL CONSULTING SERVICES.

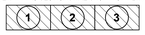
9. ALL SHOP DRAWINGS SHALL BEAR THE GENERAL CONTRACTOR'S REVIEW STAMP. SHOP DRAWINGS NOT STAMPED BY THE GENERAL CONTRACTOR WILL BE MARKED "REVISE AND RESUBMIT" AND RETURNED WITHOUT BEING REVIEWED.
10. SUBMIT ENGINEER SEALED SHOP DRAWINGS WHEN REQUIRED BY SPECIFICATIONS. SHOP DRAWINGS NOT SEALED, SIGNED, AND DATED WILL BE MARKED "REVISE AND RESUBMIT" AND RETURNED WITHOUT BEING REVIEWED.
11. DO NOT COMMENCE FABRICATION UNTIL RETURNED SHOP DRAWINGS HAVE BEEN EXAMINED.
12. PROVIDE FINAL RECORD DRAWINGS AFTER ALL CORRECTIONS ARE MADE.

QUALITY ASSURANCE — GN-004
March 8, 2022

1. COOPERATE WITH CONSULTANTS DOING GENERAL REVIEW AND WITH ALL INSPECTION AND TESTING AGENCIES. ASSIST IN THEIR WORK AND PROVIDE SAFE ACCESS TO WORK AREAS AS REQUIRED. PROVIDE REASONABLE NOTICE (NOT LESS THAN TWO WORKING DAYS OR 48 HOURS, WHICHEVER IS GREATER) PRIOR TO CONCEALING ANY WORK THAT REQUIRES INSPECTION, REVIEW, OR TESTING. SCHEDULE THIS WORK TO OCCUR DURING NORMAL BUSINESS HOURS. WORK TO BE GENERALLY COMPLETE AT THE TIME OF THE REVIEW.
2. THE STRUCTURAL CONSULTANT WILL PROVIDE PERIODIC GENERAL REVIEW OF A REPRESENTATIVE SAMPLE OF THE STRUCTURAL WORKS DESIGNED BY THE CONSULTANT TO CHECK GENERAL CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. THESE REVIEWS DO NOT ALLEVIATE THE CONTRACTOR'S RESPONSIBILITY TO IMPLEMENT AND MAINTAIN AN APPROPRIATE QUALITY CONTROL PROGRAM.
3. ENSURE THAT CONSTRUCTION REVIEW IS PROVIDED FOR ALL STRUCTURAL WORK DESIGNED BY PROFESSIONAL ENGINEERS RETAINED BY THE CONTRACTOR. CONSTRUCTION REVIEW TO BE BY THE ENGINEERS RESPONSIBLE FOR THE DESIGN, OR BY OTHER PROFESSIONAL ENGINEERS LICENSED AND INSURED IN THE PROVINCE OF ONTARIO AND DESIGNATED BY THE ENGINEER RESPONSIBLE FOR THE DESIGN. SUBMIT CONSTRUCTION REVIEW REPORTS FOR THE CONSULANT'S RECORDS UPON REQUEST.
4. MAINTAIN A QUALITY CONTROL PLAN FOR STRUCTURAL WORK AND MAKE IT AVAILABLE TO THE CONSULTANT UPON REQUEST. AT A MINIMUM, THE PLAN TO INCLUDE:
   1. NAMES OF PERSONNEL RESPONSIBLE FOR EXECUTION OF THE PLAN.
   2. MEANS AND METHODS FOR CONFIRMING MATERIAL COMPLIANCE WITH SPECIFICATIONS AND ASSOCIATED DOCUMENTATION PROCEDURES.
   3. PROGRAM FOR CONFIRMING AND DOCUMENTING COMPLIANCE OF SUB-TRADE QUALIFICATIONS AND THEIR INDIVIDUAL EMPLOYEES AND SUB-CONTRACTORS.
   4. PROCEDURES FOR REVIEWING COMPLIANCE IN THE FIELD WITH CONSTRUCTION DOCUMENTS INCLUDING DOCUMENTATION OF LOCATIONS REVIEWED, PHOTOGRAPHS TAKEN, AND TIMING FOR REVIEW. THE CONTRACTOR'S REVIEW MUST BE COMPLETED PRIOR TO REVIEW OF THE CONSULTANT.
   5. PROCEDURES FOR RECTIFYING DEFICIENCIES NOTED BY THE CONTRACTOR, CONSULTANTS AND INDEPENDENT INSPECTION AGENCIES.

EXISTING STRUCTURES — GN-006
December 22, 2021

1. TAKE ALL PRECAUTIONS NECESSARY TO PROTECT EXISTING STRUCTURES FROM DAMAGE DURING CONSTRUCTION.
2. VERIFY EXISTING DIMENSIONS AND CONDITIONS ON SITE PRIOR TO CONSTRUCTION. EXISTING CONDITIONS ARE ASSUMED. REPORT ANY VARIATIONS TO THE STRUCTURAL CONSULTANT BEFORE PROCEEDING WITH THE WORK.
3. EXISTING STRUCTURAL INFORMATION IS BASED UPON DRAWINGS PREPARED BY IAN DUNLOP AND ASSOCIATES DATED 20 APRIL 2015. (6)
4. INSTALL AND AFTERWARDS REMOVE ALL TEMPORARY SHORING AND BRACING REQUIRED TO ENSURE THE INTEGRITY OF THE EXISTING STRUCTURE DURING CONSTRUCTION.
5. ALL SHORING TO BE DESIGNED BY A PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR, LICENSED AND INSURED IN THE PROVINCE OF ONTARIO. PREPARE SHORING DRAWINGS SIGNED AND SEALED BY THAT ENGINEER.
6. SHORING INDICATED ON THE STRUCTURAL DRAWINGS DOES NOT REMOVE THE CONTRACTOR'S RESPONSIBILITY FOR DETERMINING WHERE SHORING WILL BE REQUIRED. INSTALL ALL MEMBERS AS REQUIRED, INCLUDING NEEDLING AND BRACING.
7. ASSESS CONSTRUCTION LOADS APPLIED TO EXISTING STRUCTURES. PROVIDE ADEQUATE SHORING IF THEY EXCEED THE EXISTING BUILDING DESIGN LOADS.
8. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE SHORING OPERATIONS, EXCAVATION AND OTHER CONSTRUCTION PROCEDURES
9. CONCRETE UNDERPINNING SECTIONS ARE SHOWN ON PLAN THUS:



   - A. INSTALL ALL NO. 1 SECTIONS FIRST, AND THEN FOLLOW WITH ALL NO. 2 SECTIONS, ETC.
   - B. PROVIDE 50 (2") OF DRY PACK GROUT BETWEEN EXISTING FOUNDATION AND TOP OF UNDERPINNING, NO SOONER THAN 24 HOURS AFTER UNDERPINNING IS PLACED.
   - C. ALLOW 12 HOURS AFTER GROUTING BEFORE INSTALLATION OF FOLLOWING SECTIONS.
   - D. PREVENT LOSS OF MATERIAL FROM UNDERNEATH EXISTING STRUCTURE.
   - E. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE UNDERPINNING OPERATIONS.
   - F. UNDERPINNING DETAILS AND PROCEDURE TO BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PLACE WHERE THE PROJECT IS LOCATED.
   - G. SUBMIT STAMPED DOCUMENTS FOR REVIEW BY LEA.
   - H. UNDERPINNING ENGINEER TO PROVIDE SITE REVIEW AND SUBMIT REPORTS FOR RECORD.
   - I. PREVENT LOSS OF MATERIAL FROM UNDERNEATH EXISTING STRUCTURE.
   - J. MAKE GOOD ALL EXISTING WORK DISTURBED BY THE UNDERPINNING OPERATIONS.
10. EXISTING CONCRETE STRUCTURE
   - A. REMOVE PORTIONS OF EXISTING CONCRETE STRUCTURE AS REQUIRED.
   - B. PROVIDE TEMPORARY SHORING AND BRACING AS REQUIRED.
   - C. RETAIN A PROFESSIONAL ENGINEER TO DESIGN THE TEMPORARY SHORING AND BRACING AND TO REVIEW THIS WORK ON SITE.
   - D. OBTAIN CONSULTANT'S APPROVAL BEFORE CORING OR CUTTING EXISTING SLABS, BEAMS OR WALLS.
   - E. RETAIN AN INDEPENDENT TESTING COMPANY TO LOCATE EXISTING REINFORCEMENT AND CONDUIT IN THE AREAS OF PROPOSED OPENINGS AND TO MARK LOCATIONS ON THE SURFACES OF SLABS AND WALLS ON WHICH THE CORES AND CUTS ARE TO BE STARTED USING A NON-DESTRUCTIVE METHOD.
   - F. REMOVE TOPPINGS PRIOR TO LOCATING REINFORCEMENT AND CONDUIT.
   - G. MARK LOCATIONS AND SIZES OF CORES AND OPENINGS AND LOCATIONS OF REINFORCEMENT AND CONDUIT USING INDELIBLE MARKERS AS FOLLOWS:
     - a. RED FOR TOP BARS
     - b. GREEN FOR BOTTOM BARS
     - c. BLACK FOR CORES, OPENINGS AND CONDUIT.
   - H. RELOCATE PROPOSED OPENINGS AND REPEAT PROCESS AT NO EXTRA COST TO THE CONTRACT IF PROPOSED LOCATIONS ARE NOT ACCEPTABLE TO CONSULTANT.
   - I. SAVE THE COMPLETE LENGTH OF ALL CORES. LABEL EACH CORE WITH LOCATION TAKEN. MAKE ALL CORES AVAILABLE FOR REVIEW BY CONSULTANT. DISPOSE OF CORES ONLY WITH APPROVAL OF CONSULTANT. SEE DETAILS ON STRUCTURAL DRAWINGS FOR SAWCUTTING PROCEDURE.
   - J. IF NEW REINFORCEMENT IS REQUIRED AT AN OPENING, INSTALL REINFORCEMENT BEFORE CUTTING OPENING OR SHORE UP STRUCTURE UNTIL NEW REINFORCEMENT IS INSTALLED.
   - K. ROUGHENING EXISTING SURFACES:
     - a. WHERE DRAWINGS CALL FOR A ROUGHENED SURFACE, BUSH HAMMER ENTIRE SURFACE TO A FULL AMPLITUDEOF AT LEAST 5MM.
   - L. PATCHING: PATCH EXISTING CONCRETE WHERE NECESSARY TO PROVIDE REQUIRED SMOOTH, FLAT SURFACES FOR REINFORCEMENT AND FOR OTHER TRADES.
11. EXISTING STEEL STRUCTURE
   - A. MODIFICATION / REMOVAL OF EXISTING STEEL WORK
     - a. A SET OF STRUCTURAL DRAWINGS OF THE EXISTING BUILDING MAY BE VIEWED AT THE OFFICES OF THE CONSULTANT OR THE STRUCTURAL ENGINEER.
     - b. DISMANTLE AND CUT EXISTING STRUCTURAL STEEL AS REQUIRED. PROVIDE TEMPORARY SHORING ANDBRACING REQUIRED FOR THESE OPERATIONS. RETAIN A PROFESSIONAL ENGINEER TO DESIGN THE TEMPORARY SHORING AND TO REVIEW THIS WORK ON SITE.
     - c. REMOVE FROM SITE EXISTING STEEL WHICH IS DISMANTLED BUT NOT DESIGNATED FOR REUSE. IT SHALL BECOME THE PROPERTY OF THE CONTRACTOR.
   - B. CLEAN EXISTING STRUCTURAL STEEL WHICH IS AFFECTED BY THE WORK AND IS TO REMAIN IN PLACE, DOWN TO BARE METAL, PRIOR TO ITS INSPECTION SO THAT ITS CONDITION MAY BE ASCERTAINED. NOTIFY CONSULTANT WHEN MEMBERS ARE READY FOR INSPECTION.

FOUNDATIONS — GN-007
December 22, 2021

1. STRUCTURAL DESIGN IS BASED ON THE GEOTECHNICAL REPORT PREPARED BY ENGLOBE REPORT NO. 022103870.006 DATED JANUARY 19, 2026 AND ENGLOBE GEOTECHNICAL LETTER (MASW TEST, REFERENCE NO. 022103870.007) DATED FEBRUARY 25, 2026.
2. REFER TO THE GEOTECHNICAL REPORT FOR INFORMATION ON GEOTECHNICAL CONDITIONS, FOUNDATION RECOMMENDATIONS, AND FOR ALL EARTHWORK INCLUDING EXCAVATION, BACKFILL, AND SUBGRADE PREPARATION. ALSO REFER TO ARCHITECTURAL DRAWINGS AND SPECIFICATIONS
3. CONSTRUCT ALL FOOTINGS ON STRATA CAPABLE OF SUPPORTING THE DESIGN BEARING RESISTANCE NOTED:
   - 300 kPa AT ULS (ULTIMATE LIMIT STATES DESIGN)
   - 200 kPa AT SLS (SERVICEABILITY LIMIT STATES DESIGN)

   FOR RAFT FOUNDATION THE DESIGN BEARING RESISTANCE NOTED:
   - 250 kPa AT ULS (ULTIMATE LIMIT STATES DESIGN)
   - 150 kPa AT SLS (SERVICEABILITY LIMIT STATES DESIGN)
4. FOR FROST PROTECTION, MINIMUM DISTANCE FROM FINISHED GRADE TO UNDERSIDE OF PERIMETER FOOTINGS AND FOOTINGS IN UNHEATED AREAS TO BE NOT LESS THAN 1200.
5. BEARING ELEVATIONS SHOWN ON STRUCTURAL DRAWINGS ARE ANTICIPATED MINIMUM DEPTHS. FINAL BEARING ELEVATIONS AND CAPACITY MUST BE REVIEWED AND APPROVED BY THE GEOTECHNICAL CONSULTANT PRIOR TO CONSTRUCTING FOOTINGS. ANY DEVIATIONS FROM THE CONTRACT DOCUMENTS TO BE REVIEWED WITH THE STRUCTURAL CONSULTANT PRIOR TO PLACING FORMWORK.
6. MODULUS OF SUBGRADE REACTION FOR DESIGN OF RAFT SLAB IS 6000 kPa/m FOR 25mm. MODULUS OF SUBGRADE REACTION FOR DESIGN OF SLAB ON GRADE IS 18000 kPa/m. CONSTRUCT SUBGRADE UNDER THE SLAB ON GRADE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.
7. CENTRE FOOTINGS AND PIERS UNDER CENTROID OF COLUMNS, OR WHERE THERE ARE NO COLUMNS ABOVE, CENTER UNDER WALLS OR GRADE BEAMS, UNLESS OTHERWISE NOTED.
8. UNLESS OTHERWISE NOTED, THE LONGER DIMENSION OF RECTANGULAR SPREAD FOOTINGS TO BE PARALLEL TO THE LONGER COLUMN DIMENSION.
9. LOCATE ALL EXISTING UNDERGROUND SERVICES PRIOR TO EXCAVATION.
10. THE LINE OF SLOPE BETWEEN ADJACENT EXCAVATIONS FOR FOOTINGS OR TRENCHES SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10, UNLESS OTHERWISE APPROVED BY THE GEOTECHNICAL CONSULTANT. REFER TO TYPICAL DETAILS.
11. KEEP EXCAVATION DRAINED AND FREE OF WATER AT ALL TIMES. REFER TO GEOTECHNICAL REPORT FOR REQUIRED DEWATERING PROCESS.
12. PROTECT FOOTINGS, WALLS, SLABS-ON-GRADE AND ADJACENT SOIL AGAINST FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION. DO NOT POUR CONCRETE AGAINST FROZEN EARTH.
13. DO NOT USE EARTH FORMS UNLESS APPROVED THE GEOTECHNICAL ENGINEER AND THE STRUCTURAL CONSULTANT. FOR ELEMENTS CAST AGAINST SOIL, INCREASE SIZE SHOWN ON THE DRAWINGS AS REQUIRED TO OBTAIN 75 (3") CONCRETE COVER AGAINST SOIL.
14. DO NOT BACKFILL AGAINST WALLS RETAINING EARTH UNTIL ELEMENTS PROVIDING LATERAL SUPPORT, INCLUDING SLABS ON GRADE AND SUSPENDED LEVELS, ARE COMPLETED AND CONCRETE HAS REACHED 70% OF ITS DESIGN STRENGTH.
15. FOR ELEMENTS THAT ARE TO BE BACKFILLED ON BOTH SIDES, PLACE BACKFILL SIMULTANEOUSLY ON BOTH SIDES SUCH THAT HEIGHTS DO NOT VARY BY MORE THAN 300 (1'-0") FROM ONE SIDE TO THE OTHER.
16. FORM ALL SIDES OF FOOTINGS UNLESS OTHERWISE NOTED OR APPROVED BY GEOTECHNICAL ENGINEER. INCREASE COVER TO REINFORCING.

DESIGN CRITERIA — GN-005
JANUARY 1, 2025

1. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE 2024 ONTARIO BUILDING CODE (OBC) SUPPLEMENTED BY THE 2020 NATIONAL BUILDING CODE OF CANADA STRUCTURAL COMMENTARY.
2. CLIMATIC DATA USED TO DETERMINE THE DESIGN LOADS IS FROM THE MMAH SUPPLEMENTARY STANDARD OF THE OBC SPECIFICALLY FOR TORONTO (NORTH YORK).
3. DESIGN LOADS SHOWN ON PLAN OR SECTION ARE SPECIFIED (UNFACTORED) LOADS TO BE USED FOR ULS DESIGN, UNLESS OTHERWISE NOTED. FOR POINT LOADS, CONSIDER IT A LIVE LOAD IF ONLY ONE LOAD IS SHOWN. LOAD FACTORS TO CONVERT LOADS TO SLS FOR WIND AND SNOW ARE SHOWN IN THEIR RESPECTIVE SECTIONS.
4. SELF WEIGHT IS DUE TO THE WEIGHT OF THE STRUCTURE ITSELF. IT VARIES WITH THE STRUCTURAL SYSTEM AND INCLUDES CONCRETE TOPPINGS ON STEEL DECK.
5. SUPERIMPOSED DEAD LOADS (SDL) ARE NON-STRUCTURAL DEAD LOADS DUE TO NON-STRUCTURAL TOPPINGS, FINISHES, PARTITIONS, ROOFING MATERIALS, SUSPENDED EQUIPMENT, PAVERS, SOIL, ETC.
6. DEAD LOAD (DL) IS THE SELF WEIGHT OF THE STRUCTURE PLUS THE SUPERIMPOSED DEAD LOAD.
7. GRAVITY LOADS ARE SHOWN ON PLANS. LIVE LOAD REDUCTION HAS NOT BEEN USED.
8. NET FACTORED UPLIFT LOADS ON ROOFS ARE SHOWN ON PLANS.
9. FIRE
   - A. STRUCTURAL DESIGN ASSUMES NON-LOAD RESTRICTED ULC FIRE RATED ASSEMBLIES, AND APPROPRIATE MATERIALS MUST BE USED
10. IMPORTANCE: HIGH
11. **LOADS DUE TO SNOW AND RAIN**

SNOW AND ASSOCIATED RAIN (S) – OBC CLAUSE 4.1.6.2
MINIMUM SPECIFIED SNOW LOAD, $S=I_sS_s(S_a(C_bC_wC_sC_a)+S_r)= 1.56$ kN/m²

IMPORTANCE FACTOR =	I_s =	ULS = 1.15 SLS = 0.9	
GROUND SNOW LOAD (1-IN-50 YEAR)	S_s =	1.20 kN/m²	
BASIC ROOF FACTOR =	C_b =	0.80	
WIND EXPOSURE FACTOR =	C_w =	1.0	
SLOPE FACTOR =	C_s =	1.0,(MIN)	SEE PLAN FOR ROOF SPECIFIC LOADS
ACCUMULATION FACTOR =	C_a =	1.0,(MIN)	SEE PLAN FOR ROOF SPECIFIC LOADS
ASSOCIATED RAIN LOAD =	S_r =	0.4 kN/m²	

RAIN (S) AND ROOF DRAINAGE– OBC CLAUSE 4.1.6.4
24 HOUR RAINFALL = 108 mm
ROOFS HAVE BEEN DESIGNED FOR THE WORST CASE OF SNOW OR RAIN, AS PER OBC 4.1.6.4(3) & (4).
ROOF DRAINAGE SYSTEM:
1) RAIN LOAD IS DETERMINED FOR FLOW CONTROL ROOF DRAINS THAT MEET THE REQUIREMENTS OBC 4.1.6.4 (4) AND OBC 7.4.10.4, INCLUDING EMERGENCY SCUPPERS AT 150mm ABOVE THE HIGH POINT OF THE ROOF. THE ROOF STRUCTURE MUST INCLUDE THE LOAD ACCUMULATED BY A 24H RAINFALL, BUT RAIN AND SNOW LOADS DO NOT NEED TO BE CONSIDERED TO ACT SIMULTANEOUSLY TO A MAXIMUM OF 180mm ACCUMULATED WATER.

12. **LATERAL LOADS**

LATERAL FORCE RESISTING SYSTEM:
- CONVENTIONAL CONSTRUCTION CONCRETE SHEAR WALLS WITH HORIZONTAL CONCRETE SLAB DIAPHRAGM
- CONVENTIONAL CONSTRUCTION STEEL BRACED FRAME WITH HORIZONTAL CONCRETE ON STEEL DECK
- CONVENTIONAL CONSTRUCTION STEEL BRACED FRAME WITH HORIZONTAL CONCRETE ON STEEL DECK AND HORIZONTAL BRACING

WIND (W) – OBC 4.1.7.1- STATIC PROCEDURE
MINIMUM SPECIFIED BUILDING WIND LOAD AT GRADE, $P=I_wq_hC_eC_gC_p= 0.94$ kPa
INTERNAL PRESSURE: CATEGORY 2 (OBC 4.1.7.7)

IMPORTANCE FACTOR =	I_w =	ULS = 1.15 SLS = 0.75	OBC 4.1.7.3.
REFERENCE VELOCITY PRESSURE (1-IN-50 YEAR)	q =	0.44 kPa	OBC 4.1.7.3.
EXPOSURE FACTOR (TOP OF BUILDING) =	C_e =	1.04 (MAX)	OBC 4.1.7.3. OR ***4.1.7.8***
TOPOGRAPHIC FACTOR =	C_t =	1.0	OBC 4.1.7.4.
GUST EFFECT FACTOR =	C_g =	2.0	OBC 4.1.7.3. OR ***4.1.7.8***
EXTERNAL PRESSURE COEFFICIENT (WINDWARD)	C_p =	0.621	OBC 4.1.7.5.
EXTERNAL PRESSURE COEFFICIENT (LEEWARD)	C_p =	0.319	OBC 4.1.7.5.
FACTORED SHEAR TO LATERAL LOAD RESISTING SYSTEM(NORTH-SOUTH)	V_{W_NS} =	400 kN	
FACTORED SHEAR TO LATERAL LOAD RESISTING SYSTEM (EAST-WEST)	V_{W_EW} =	425 kN	

EARTHQUAKE LOADS AND EFFECTS (E) - OBC 4.1.8
SEISMIC PARAMETERS

IMPORTANCE FACTOR =	I_E =	1.30	OBC 4.1.8.5
PROBABILITY OF EXCEEDANCE (IN 50 YEARS)		2%	4.1.8.4 (1)

SITE PROPERTIE

AVERAGE SHEAR WAVE VELOCITY =	V_{s30} =	453 m/s	OBC 4.1.8.4 (2)
SITE CLASS	S =	C	OBC 4.1.8.4 (3)
SITE DESIGNATION	X	X_C	
PEAK GROUND ACCELERATION	PGA(X)	0.154	OBC 4.1.8.4 (3)
PEAK GROUND VELOCITY	PGV(X)	0.109	OBC 4.1.8.4 (3)

2% PROBABILITY OF EXCEEDANCE (5% DAMPED) SPECTRAL ACCELERATION VALUES

PERIOD, T (SECONDS)	S(T, X)	DESIGN SPECTRAL ACCELERATION, S(T)
<=0.2	Sa(0.2, X) =	0.310
0.5	Sa(0.5, X) =	0.166
1.0	Sa(1.0, X) =	0.0863
2.0	Sa(2.0, X) =	0.0396
5.0	Sa(5.0, X) =	0.0103
10.0	Sa(10.0, X) =	0.00357

10% PROBABILITY OF EXCEEDANCE (5% DAMPED) SPECTRAL ACCELERATION VALUES

PERIOD, T (SECONDS)	S(T, X)	DESIGN SPECTRAL ACCELERATION, S(T)
<=0.2	Sa(0.2, X) =	0.102
0.5	Sa(0.5, X) =	0.0586
1.0	Sa(1.0, X) =	0.0299
2.0	Sa(2.0, X) =	0.0131
5.0	Sa(5.0, X) =	0.003
10.0	Sa(10.0, X) =	0.00107

SEISMIC LOAD AND PERFORMANCE

$I_E S(0.2)$ =	0.403		OBC 4.1.8.5
$I_E S(1.0)$ =	0.1122		OBC 4.1.8.5
SEISMIC CATEGORY	SC3		OBC 4.1.8.5
IRREGULARITIES	NOT APPLICABLE		OBC 4.1.8.9
METHOD OF ANALYSIS	STATIC	STRUCTURE HEIGHT < 60m AND Ta < 2s	OBC 4.1.8.7

		BUILDING	
DIRECTION		**NORTH-SOUTH**	**EAST-WEST**
SFRS DUCTILITY RELATED FORCE MODIFICATION FACTORS	R_d =	1.5	1.5
SFRS OVERSTRENGTH RELATED FORCE MODIFICATION FACTORS	R_O =	1.3	1.3
FUNDAMENTAL LATERAL PERIOD	Ta =	0.31 s	0.31 s
MAX. HIGHER MODE FACTOR	M_v =	1.0	1.0
TORSIONAL SENSITIVITY	B =	<=1.7	<=1.7
DESIGN BASE SHEAR	V = MAX BETWEEN 1. (2/3) x S(0.2) x I_E x W / (R_d x R_o) 2. S(0.5) I_E x W / (R_d x R_o)	2700 kN	2700 kN
	V = $S(T_a)$ x M_v x I_E x W / (R_d x R_o)	2960 kN	2960 kN
	V_{MIN} = S(4.0) x M_v x I_E x W / (R_d x R_o)	185 kN	185 kN

MAX. INTERSTOREY DRIFT RATIO: 0.005xHs = 60mm USING Ie=1.0 AND RdRo=1.0
FOR 10% PROBABILITY OF EXCEEDANCE SPECTRAL VALUES

CONCRETE — GN-008
December 22, 2021

1. CONFORM TO CSA A23.1: "CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION".
2. CONCRETE IS SPECIFIED PER ALTERNATIVE 1 - PERFORMANCE SPECIFICATION AS OUTLINED IN CAN/CSA A23.1. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO MEET ALL CERTIFICATION, DOCUMENTATION, AND QUALITY CONTROL REQUIREMENTS.
3. THE CONCRETE SUPPLIER TO BE CERTIFIED BY "CONCRETE ONTARIO" (FORMERLY RMCAO CERTIFICATION).
4. THE CONTRACTOR AND THE CONCRETE SUPPLIER TO ENSURE THAT THE PLASTIC AND HARDENED MIX PROPERTIES MEET SITE REQUIREMENTS FOR PLACING, FINISHING, AND THE SPECIFIED PERFORMANCE REQUIREMENTS.
5. CEMENT TO BE PORTLAND CEMENT TYPE GU UNLESS NOTED OTHERWISE OR REQUIRED BY EXPOSURE CLASS.
6. CONCRETE TO BE NORMAL DENSITY, UNLESS NOTED OTHERWISE.
7. NOMINAL SIZE OF COURSE AGGREGATE TO BE 20 (3/4"), UNLESS NOTED OTHERWISE.
8. PLACE CONCRETE AS CLOSE AS POSSIBLE TO FINAL LOCATION TO AVOID SEGREGATION. VIBRATE ALL CONCRETE.
9. PROTECT CONCRETE FROM FREEZING. DO NOT PLACE CONCRETE AGAINST FROZEN GROUND. USE COLD WEATHER CONCRETING METHODS IN ACCORDANCE WITH CAN/CSA-A23.1.
10. PROTECT CONCRET

CONCRETE COVER TO REINFORCING STEEL

GN-009
December 22, 2021

CONCRETE COVER TABLE WITH 2 HOUR FIRE RATING, NOT PRESTRESSED

STRUCTURAL ELEMENT	BAR SIZE (M)	BUILDING INTERIOR (WITHIN VAPOUR BARRIER) BELOW GRADE (EXCLUDE PARKING STRUCTURE) COVER (mm)		
		TOP	BOTTOM	SIDE
FOOTING (WITHOUT FORMWORK)	≤ 35	50	75	75
FOOTING (WITH FORMWORK)	≤ 35	50	75	50
GRADE BEAM AND PIER (WITH FORMWORK / LINER)	≤ 35	50	50	50
COLUMN (TO TIE)	≤ 35			40
SHEAR WALL	≤ 25			25
	≤ 30			30
	≤ 35			35
BEAM (TO STIRRUP)	≤ 35	30	30	30
PERIMETER FOUNDATION WALL (EXTERIOR FACE REINFORCEMENT - CAST AGAINST EARTH)	≤ 35			75
PERIMETER FOUNDATION WALL (EXTERIOR FACE NOT CAST AGAINST EARTH + INTERIOR FACE)	≤ 25			40
	≤ 30			45
	≤ 35			55
BELOW GRADE SLAB	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	

STRUCTURAL ELEMENT	BAR SIZE (M)	INTERIOR (WITHIN VAPOUR BARRIER) ABOVE GRADE (EXCLUDE PARKING STRUCTURE) COVER (mm)		
		TOP	BOTTOM	SIDE
SHEAR WALL EXPOSED TO FIRE ON BOTH SIDES (SEE PLAN OR ELEVATION)	≤ 35			50
GROUND FLOOR SLAB (NON VEHICLE ACCESSIBLE)	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	
FLOOR SLAB (EXCLUDING EXPOSED BALCONY SLAB)	≤ 25	25	25	
	≤ 30	30	30	
	≤ 35	35	35	
STRUCTURAL ELEMENT	BAR SIZE (M)	EXTERIOR (OUTSIDE VAPOUR BARRIER) ABOVE GRADE COVER (mm)		
		TOP	BOTTOM	SIDE
COLUMN (TO TIE)	≤ 35			60
COLUMN (TO TIE)	≤ 30			40
	≤ 35			45
SHEAR WALL	≤ 30			60
	≤ 35			70
SHEAR WALL	≤ 25			40
	≤ 30			45
	≤ 35			55
RETAINING WALL	≤ 30			60
	≤ 35			70
RETAINING WALL	≤ 25			40
	≤ 30			45
	≤ 35			55
SLAB	≤ 30	60	60	
	≤ 35	70	70	
SLAB / EXPOSED BALCONY SLAB	≤ 25	40	40	
	≤ 30	45	45	
	≤ 35	55	55	
BEAM (TO STIRRUP)	≤ 35	60	60	60
BEAM (TO STIRRUP)	≤ 25	30	30	30
	≤ 30	35	35	35
	≤ 35	45	45	45

* REFER TO PLAN FOR LOCATION WHERE DIFFERENT CONCRETE COVER IS REQUIRED.

CONCRETE REINFORCEMENT

GN-009A
December 22, 2021

1. REINFORCEMENT IS TO CONFORM TO THE FOLLOWING STANDARDS:
   - A. DEFORMED BARS: CAN/CSA G30.18, GRADE 400R.
   - B. WELDED DEFORMED BARS: CAN/CSA G30.18, GRADE 400W.
   - C. WELDED WIRE FABRIC: ASTM A185/A185M.
   - D. EPOXY COATING: ASTM D3963
2. PROVIDE MIN. 24 HOURS NOTICE PRIOR TO CLOSING FORMS OR PLACING CONCRETE TO ALLOW FOR REINFORCING STEEL REVIEW.
3. BARS MARKED CONTINUOUS TO BE TERMINATED IN STANDARD HOOKS AT ENDS AND SPLICED USING CLASS B LAPS. DO NOT SHORTEN HOOKS TO ACCOMMODATE MEMBER DIMENSION.
4. ALL REBAR HOOKS TO BE STANDARD LENGTH 90° OR 180° HOOKS. REBAR LENGTHS LISTED ON DRAWINGS DO NOT INCLUDE THE HOOK LENGTH.
5. WHERE TWO BARS OF DIFFERENT SIZE ARE LAPPED IN TENSION, SPLICE LENGTH TO BE EQUAL TO THE SMALLER BAR'S TENSION LAP SPLICE, OR TO THE LARGER BAR'S TENSION DEVELOPMENT LENGTH, WHICHEVER IS LONGER.
6. WHERE TWO BARS OF DIFFERENT SIZE ARE LAPPED IN COMPRESSION, SPLICE LENGTH TO BE EQUAL TO THE SMALLER BAR'S COMPRESSION LAP SPLICE, OR TO THE LARGER BAR'S COMPRESSION DEVELOPMENT LENGTH, WHICHEVER IS LONGER.
7. PROVIDE ADDITIONAL SUPPORT BARS AS REQUIRED TO ADEQUATELY SUPPORT AND SECURE ALL REINFORCEMENT AND PREVENT MOVEMENT WHEN PLACING CONCRETE.
8. PROVIDE SUFFICIENT CHAIRS TO REINFORCING TO MAINTAIN SPECIFIED CONCRETE COVER.
9. ALL REINFORCING TO BE CLEAN, FREE OF LOOSE SCALE, OIL, DIRT, RUST, AND ANY OTHER FOREIGN COATING THAT AFFECT BONDING CAPACITY.
10. MINIMUM CLEAR CONCRETE COVER TO REINFORCEMENT CLOSEST TO THE SURFACE TO BE AS FOLLOWS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
11. WALLS
    - A. UNLESS OTHERWISE NOTED ON DRAWINGS, SCHEDULES OR NOTES, MINIMUM REINFORCEMENT FOR CONCRETE WALLS TO BE AS FOLLOWS:
      - a. 150 MAXIMUM WALL: 10 @ 300 (12") H + 10 @ 400 (16") V IN CENTRE
      - b. 200 MAXIMUM WALL: 10 @ 250 (10") H + 10 @ 300 (12") V IN CENTRE
      - c. 250 MAXIMUM WALL: 10 @ 400 (16") HEF + 10 @ 500 (20") VEF
      - d. 300 MAXIMUM WALL: 10 @ 330 (13") HEF + 10 @ 440 (17") VEF
      - e. VERTICAL REINFORCING TO BE OUTSIDE LAYER.
    - B. REFER TO TYPICAL DETAILS FOR REINFORCING REQUIRED AT WALL CORNERS, INTERSECTIONS AND OPENINGS, AND FOR MINIMUM DOWELING REQUIREMENTS BETWEEN WALLS AND SLABS.
12. SLABS
    - A. UNLESS OTHERWISE SPECIFIED ON PLANS, PROVIDE BOTTOM TEMPERATURE REINFORCEMENT IN BOTH DIRECTIONS FOR FRAMED SLABS IN ACCORDANCE WITH TYPICAL DETAILS.
    - B. DO NOT ELIMINATE, CUT, OR DISPLACE REINFORCING TO ACCOMMODATE MECHANICAL AND ELECTRICAL SLEEVES, OPENINGS OR HARDWARE. SPREAD REINFORCING AROUND SLEEVES. PROVIDE ADDITIONAL REINFORCING AS SHOWN IN TYPICAL DETAILS AND DRAWINGS FOROPENINGS LARGER THAN 450 x 450 (18"X18"). IF OPENINGS, SLEEVES, OR HARDWARE CANNOT BE LOCATED AS SPECIFIED, OBTAIN APPROVAL OF MODIFICATIONS FROM CONSULTANT PRIOR TO PLACING CONCRETE.

CONDUITS AND EMBEDS

GN-010
December 22, 2021

1. GENERAL:
   - A. ALL CONCRETE EMBEDS (PIPES, CONDUITS, SLEEVES, ETC. INCLUDING ANY EMBEDDED FITTINGS OR COUPLERS) AND THEIR INSTALLATION TO CONFORM TO CSA A23.1/A23.2 AND THE FOLLOWING GUIDELINES. THE MORE STRINGENT GUIDELINES GOVERN.
   - B. ALL CONCRETE EMBEDS SHALL NOT IMPAIR THE STRENGTH OF STRUCTURAL ELEMENTS AND SHALL BE MOVED OR REMOVED IF DIRECTED BY THE CONSULTANT. NO CONCRETE EMBEDS ARE PERMITTED WITHIN "PROTECTED ZONES" INDICATED ON THE STRUCTURAL DRAWINGS. IF REQUIRED, REMEDIAL REINFORCING AT ZONES OF CONGESTED EMBEDS SHALL BE PROVIDED AT NO EXTRA COST TO THE CONTRACT.
   - C. DO NOT CUT, ELIMINATE OR DISPLACE REINFORCING TO INSTALL EMBEDS OR HARDWARE UNLESS APPROVED IN WRITING BY THE CONSULTANT. OBTAIN APPROVAL BEFORE PLACING CONCRETE.
   - D. WHERE APPROVED BY THE CONSULTANT, SET SLEEVES, TIES, PIPE HANGERS AND OTHER INSERTS AND OPENINGS AS INDICATED OR SPECIFIED ELSEWHERE. SLEEVES AND OPENINGS GREATER THAN 100 X 100 mm (4" X 4") NOT INDICATED MUST BE REVIEWED BY THE CONSULTANT.
   - E. CONCRETE EMBEDS SHALL NOT HAVE AN OUTSIDE DIAMETER LARGER THAN ⅓ OF THE THICKNESS OF CONCRETE ELEMENT WHICH THEY ARE EMBEDDED IN AND SHALL NOT BE SPACED CLOSER THAN 3 DIAMETERS ON CENTER. THE LARGER EMBED DIAMETER WILL GOVERN FOR SPACING.
   - F. EMBEDS WITHIN A CONCRETE ELEMENT WILL BE PLACED WITHIN THE MIDDLE THIRD OF THE ELEMENT THICKNESS. PROVIDE CHAIRS/SUPPORTS FOR THE EMBEDS. DO NOT TIE TO REINFORCING STEEL. PROVIDE MINIMUM 50 CLEAR SPACE BETWEEN ALL EMBEDS AND REINFORCING BARS.
   - G. SUBMIT A COMBINED LAYOUT DRAWING SET SHOWING SLEEVES AND OPENINGS REQUIRED BY ALL TRADES FOR THE CONSULTANT'S REVIEW PRIOR TO CONSTRUCTION. SLEEVE COORDINATION TO BE DONE BY OTHERS. CONFIRM LOCATIONS AND SIZES OF SLEEVES AND OPENINGS SHOWN ON DRAWINGS.
   - H. METAL EMBEDS OTHER THAN CARBON STEEL MUST BE ELECTRICALLY ISOLATED WITH A DIELECTRIC COATING.
2. CONCRETE VERTICAL ELEMENTS:
   - A. DO NOT PLACE EMBEDS INCLUDING CONDUITS, SLEEVES AND RECESSED BOXES PERMITTED IN ANY COLUMN WITHOUT PRIOR APPROVAL FROM THE CONSULTANT.
   - B. CONCRETE WALL EMBEDS TO BE SPACED MINIMUM AT 4 DIAMETERS ON CENTER. VERTICAL EMBEDS TO BE LOCATED WITHIN MIDDLE THIRD OF WALL AND TO HAVE MINIMUM 75 CLEAR COVER. NO HORIZONTAL EMBED LENGTHS PERMITTED ALONG THE LONG DIRECTION WITHIN ANY WALL ONLY VERTICAL EMBEDS AND THROUGH WALLS SLEEVES ARE PERMITTED.
   - C. NO EMBEDS PERMITTED WITHIN WALL CONCENTRATED ZONE STEEL AREAS OR WITHIN 1000 OF WALL END.
3. CONCRETE HORIZONTAL ELEMENTS:
   - A. MAXIMUM 2 LAYERS OF CONDUITS PERMITTED WITHIN A CONCRETE SLAB, INCLUDING CROSSOVERS.
   - B. PENETRATIONS TO BE SET A MINIMUM OF 3X SLAB THICKNESS AWAY FROM THE FACE OF ANY COLUMN. DO NOT LOCATE PENETRATIONS WITHIN COLUMN CAPITALS OR DROP PANELS. PENETRATIONS TO BE SET A MINIMUM OF 3X SLAB THICKNESS AWAY FROM THE EDGES OF DROP PANELS OR STUD RAILS. SLEEVES TO NOT BE LOCATED WITHIN COLUMN ZONES WITHOUT WRITTEN APPROVAL FROM THE CONSULTANT.
   - C. MAXIMUM OUTSIDE DIAMETER OF ANY SLEEVE, CONDUIT OR EMBED IN OR THROUGH THE SLAB NOT TO EXCEED ONE QUARTER OF THE SLAB THICKNESS AND MINIMUM SPACING TO BE 3 DIAMETERS ON CENTRE ALL AROUND.
   - D. NO SLEEVES THROUGH ANY BEAMS OR SLAB BANDS UNLESS APPROVED IN WRITING BY THE CONSULTANT.
   - E. SLEEVES AND EMBEDS ARE NOT PERMITTED IN BONDED TOPPINGS UNLESS APPROVED BY THE CONSULTANT.
   - F. FOR SLABS-ON-GRADE OR UNBONDED CONCRETE TOPPINGS, LOCATE ALL CONDUITS, PIPES, OR HEATING CABLES EMBEDDED IN CONCRETE CLEAR OF THE TOP ONE THIRD OF THE THICKNESS TO AVOID DAMAGE DURING SAWCUTTING.
   - G. PT SLABS TO FOLLOW SAME RULES AS NORMAL SLABS. IN ADDITION, NO EMBEDS SHALL BE PLACED NEAR POST-TENSIONING ANCHORAGES UNLESS APPROVED BY THE PT CONTRACTOR.

POST-INSTALLED ANCHORS AND DOWELS

GN-011
December 22, 2021

1. UNLESS OTHERWISE NOTED, ANCHORAGE TO CONCRETE TO BE:
   - A. WHERE DRILLED CONCRETE ANCHORS (DCA) ARE NOTED ON DRAWINGS, PROVIDE HILTI KWIK BOLT – TZ2 EXPANSION ANCHORS.
   - B. WHERE ADHESIVE ANCHORS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM WITH HILTI HIT-Z ANCHOR RODS USING HILTI SAFESET HOLLOW DRILL BIT TECHNOLOGY.
   - C. WHERE DRILLED REINFORCING STEEL DOWELS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 200 ADHESIVE ANCHORING SYSTEM INSTALLED USING HILTI SAFESET HOLLOW DRILL BIT TECHNOLOGY.
2. UNLESS OTHERWISE NOTED ON DRAWINGS, ANCHORAGE TO MASONRY TO BE:
   - A. FOR SOLID OR GROUTED MASONRY, WHERE DRILLED MASONRY ANCHORS (DMA) ARE NOTED ON DRAWINGS, PROVIDE HILTI KB-3 EXPANSION ANCHORS.
   - B. FOR SOLID OR GROUTED MASONRY, WHERE ADHESIVE ANCHORS ARE NOTED ON DRAWINGS, PROVIDE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HAS-E THREADED RODS.
   - C. FOR HOLLOW MASONRY, PROVIDE HILTI HIT-HY 270 ADHESIVE ANCHORING SYSTEM WITH HIT-SC MESH SLEEVE AND HAS-E THREADED RODS.
3. WHERE GALVANIZED ANCHORS ARE SPECIFIED, PROVIDE HOT DIPPED GALVANIZED (NOT ZINC PLATED) ANCHORS. Check if galvanized is available for the anchors specified. HAS-E3 TZ are currently not available galvanized.
4. CONCRETE TO BE MINIMUM 28 DAYS OLD AT THE TIME OF ANCHOR INSTALLATION.
5. USE DRILLING AND INSTALLATION TOOLS AND PROCEDURES PER MANUFACTURERS' RECOMMENDATIONS. HOLE DIAMETERS NOT TO EXCEED THOSE REQUIRED BY MANUFACTURER.
6. ARRANGE FOR A MANUFACTURER'S TECHNICAL REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL THE PRODUCTS SPECIFIED, AND CONDITIONS ENCOUNTERED (E.G. HORIZONTAL, INCLINED, OVERHEAD). SUBMIT RECORD OF TRAINING LISTING ALL TRAINED PERSONNEL. IN ADDITION, PERSONNEL INSTALLING ADHESIVE ANCHORS IN A HORIZONTALLY OR UPWARDLY INCLINED DIRECTION THAT ARE REQUIRED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE CERTIFIED IN ACCORDANCE WITH ACI/CRSI ADHESIVE INSTALLER CERTIFICATION PROGRAM AS PER CSA A23.3 ANNEX D.
7. ARRANGE FOR A MANUFACTURER'S TECHNICAL REPRESENTATIVE TO BE PRESENT DURING INSTALLATION OF FIRST FEW ANCHORS OF EACH SIZE AND TYPE. SUBMIT SITE REPORTS BY MANUFACTURER TO CONSULTANT WITHIN ONE WEEK OF EACH VISIT. REPORTS TO INDICATE ANCHOR TYPES AND SIZES INSTALLED, LOCATIONS AND INSTALLERS' NAMES.
8. DO NOT CUT REINFORCEMENT TO ACCOMMODATE DRILLED ANCHORS AND DOWELS. SCAN THE STRUCTURE TO LOCATE REINFORCEMENT PRIOR TO FABRICATING STRUCTURAL STEEL FASTENED BY DRILLED ANCHORS.
9. WHEN OBSTRUCTIONS PREVENT DRILLING HOLES IN SPECIFIED LOCATIONS TO THE REQUIRED DEPTH, RELOCATE AT NO EXTRA COST TO THE CONTRACT. OBTAIN CONSULTANT'S APPROVAL OF NEW LOCATIONS BEFORE DRILLING HOLES. FILL ALL ABANDONED HOLES WITH MIN. 30MPa GROUT. DO NOT TIGHTEN ANCHORS UNTIL GROUT IN ADJACENT ABANDONED HOLES REACHES 75% fc.
10. DO NOT BEND POST INSTALLED DOWELS AND RODS AFTER INSTALLATION.
11. STANDARD EMBEDMENT LENGTHS FOR POST-INSTALLED ANCHORS:

ANCHOR TYPE	HILTI KB-TZ2		ADHESIVE HY-200V3		ADHESIVE HY-270 +HIT-S SCREEN	
BASE MATERIAL	CONCRETE	GROUTED MASONRY	CONCRETE	GROUTED MASONRY	GROUTED MASONRY	HOLLOW MASONRY
12 (1/2")	64 (2 1/2")	64 (2 1/2")	114 (4 1/2")	114 (4 1/2")	86 (3 3/8")	51 (2")
16 (5/8")	83 (3 1/4")	83 (3 1/4")	143 (5 5/8")	143 (5 5/8")	114 (4 1/2")	51 (2")
19 (3/4")	121 (4 3/4")	102 (4")	171 (6 3/4")	171 (6 3/4")	143 (5 5/8")	51 (2")
25 (1")	N/A	N/A	229 (9")	N/A	171 (6 3/4")	51 (2")

NOTES:

1. ALL ANCHORS TO HAVE MINIMUM EFFECTIVE EMBEDMENT LENGTHS NOTED IN THE TABLE ABOVE, UNLESS OTHERWISE SHOWN ON DRAWINGS.
2. FOLLOW INSTALLATION PROCEDURE AND USE TOOLS SPECIFIED IN THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
3. GROUTED MASONRY
   - A. EMBEDMENTS SHOWN ARE FOR ANCHORS INTO THE SIDE OF A CONCRETE MASONRY UNIT.
   - B. HIT-HY 200 ANCHORS MAY BE INSTALLED IN THE SIDE OF MASONRY AT ANY LOCATIONS LIMITED TO ONE ANCHOR PER CELL.
   - C. ONLY INSTALL 5/8" AND 3/4" USING HIT HY-200V3 ANCHORS INTO GROUTED WALLS GREATER OR EQUAL TO 190 THICK.
   - D. HILTI HIT KB-TZ2 ANCHORS MAY ONLY BE INSTALLED ONLY THROUGH THE FACE SHELL OF THE BLOCK, A MINIMUM OF 35 (1-3/8") FROM ANY VERTICAL MORTAR JOINT.
4. RELOCATE ANCHORS AS REQUIRED TO CLEAR REINFORCING BARS. FILL ABANDONED ANCHOR HOLES WITH MIN. 30 MPa GROUT. DO NOT TIGHTEN ADJACENT ANCHORS UNTIL GROUT IN ABANDONED HOLES REACHES 75% fc'. DO NOT CUT ANY REINFORCING STEEL.

TEMPORARY SHORING FOR EXCAVATION

GN-020
December 22, 2021

1. READ IN CONJUNCTION WITH GENERAL NOTES, TYPICAL DETAILS, AND FLOOR PLAN NOTES. REFER ALSO TO CIVIL AND ARCHITECTURAL DRAWINGS FOR SITE INFORMATION.
2. THE TEMPORARY EXCAVATION SUPPORT SYSTEM (SHORING) IF SHOWN ON THE DRAWINGS IS SCHEMATIC ONLY. RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO TO CARRY OUT THE DESIGN OF THE SHORING SYSTEM.
3. THE CONTRACTOR IS RESPONSIBLE FOR THE SELECTION, DESIGN, AND CONSTRUCTION OF THE MOST APPROPRIATE TYPE OF TEMPORARY SUPPORT SYSTEM TO MEET THE EXISTING SITE CONDITIONS, ANTICIPATED SUBSURFACE CONDITIONS AND ANY CONSTRUCTION REQUIREMENTS.
4. THE CONTRACTOR SHALL MAKE THEIR OWN ASSESSMENT OF THE IMPACTS TO THE DESIGN OR CONSTRUCTION METHOD DUE TO EXISTING CONDITIONS. EXISTING CONDITIONS SHALL INCLUDE ADJACENT PROPERTY, STRUCTURES, OR UTILITIES. THE SELECTION AND DESIGN OF THE TEMPORARY WORKS WILL INCLUDE METHODS AND DETAILS THAT WILL ASSURE SAFETY TO THE CONSTRUCTION OPERATION, THE PUBLIC, ADJACENT PROPERTY AND STRUCTURES AND TO THE FINAL PERMANENT STRUCTURE. THE DESIGN OF THE TEMPORARY WORKS IS ALSO SUBJECT TO THE APPROVAL OF THE MUNICIPALITY HAVING JURISDICTION FOR THE PROJECT AND UTILITY COMPANIES.
5. SUBMIT SIGNED AND SEALED SHORING SHOP DRAWINGS BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN OF THE SHORING SYSTEM. THE PROFESSIONAL ENGINEER RETAINED BY THE CONTRACTOR SHALL BE REGISTERED IN THE PROVINCE OF ONTARIO.
6. THE DESIGN, FABRICATION AND INSTALLATION OF THE SHORING SYSTEM SHALL BE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE IN FORCE AT THE TIME OF PERMIT APPLICATION. ALL EXCAVATIONS SHALL COMPLY WITH THE ONTARIO HEALTH AND SAFETY ACT AND REGULATIONS FOR CONSTRUCTION PROJECTS IN FORCE AT THE TIME OF CONSTRUCTION.
7. DESIGN OF THE SHORING SHALL CONFORM TO THE LATEST EDITION OF THE CANADIAN FOUNDATION ENGINEERING MANUAL AND THE SITE SPECIFIC GEOTECHNICAL REPORT.
8. REFER TO THE GEOTECHNICAL REPORT.
9. PILE SIZE AND LAYOUT SHALL ACCOUNT FOR CONSTRUCTION TOLERANCES AND ANTICIPATED MOVEMENTS AS TO NOT ENCROACH INTO THE PERMANENT STRUCTURE EXCEPT AS NOTED ON THE DRAWINGS.
10. VERIFY ALL EXISTING STRUCTURES, SERVICES, AND UTILITIES PRIOR TO COMMENCING CONSTRUCTION.

CLADDING

GN-021
Febraury 11, 2025

1. CURTAIN WALL GRAVITY LOAD SHALL BE SUPPORTED AT EVERY FLOOR OR EVERY OTHER FLOOR; PRECAST CLADDING GRAVITY LOAD SHALL BE SUPPORTED AT EVERY FLOOR.
2. CLADDING LATERAL SUPPORT CONNECTION TO STRUCTURE SHALL BE DESIGNED TO ALLOW FOR DIFFERENTIAL VERTICAL MOVEMENT OF 25mm (MIN.)
3. NOTATIONS
   - A. CWS - CLADDING SELF-WEIGHT SUPPORTED AT THIS LEVEL
   - B. CLS - CLADDING TO HAVE LATERAL SUPPORT ONLY AT THIS LEVEL
   - C. CSS - CLADDING SUPPORTED ON CLADDING AT THIS LEVEL

STRUCTURAL STEEL

GN-012
December 22, 2021

1. CONFORM TO CSA S16: "LIMIT STATES DESIGN OF STEEL STRUCTURES".
2. FABRICATOR SHALL BE CERTIFIED BY CANADIAN WELDING BUREAU UNDER REQUIREMENTS OF CSA W47.1, DIVISION 1 OR 2
3. MATERIALS: TO CSA G40.21 UNLESS OTHERWISE NOTED, WITH THE FOLLOWING GRADES:
   - A. WIDE FLANGE SECTIONS - 350W
   - B. CHANNELS, ANGLES - 350W
   - C. PLATES, BARS - 300W
   - D. HOLLOW STRUCTURAL SECTIONS (HSS) - 350W (CLASS 'C') OR ASTM A500, 345MPa (GRADE 'C')
   - E. GALVANIZED HSS 350W (CLASS 'H') OR STRESS-RELIEVED PRIOR TO GALVANIZING.
   - F. STEEL PIPE - ASTM A53, 240W
   - G. STRUCTURAL STEEL BOLTS - ASTM A325M, UNLESS NOTED
   - H. ANCHOR RODS - 300W
   - I. SPECIAL ANCHOR RODS - ASTM F1554 GRADE 105 (WHERE SHOWN ON DRAWINGS)
4. DETAILS ON STRUCTURAL DRAWINGS SHOW DESIGN INTENT. REFER TO SPECIFICATIONS FOR CONNECTION DESIGN, DETAILING, FABRICATION, AND ERECTION REQUIREMENTS.
5. DO NOT CUT HOLES OR OTHERWISE MODIFY STRUCTURAL MEMBERS ON SITE.
6. CLEAN SURFACES DOWN TO BARE METAL AND APPLY TWO COATS OF ZINC-RICH TOUCH-UP PAINT TO ANY GALVANIZED SURFACE THAT HAS BEEN DAMAGED OR FIELD WELDED.
7. PROVIDE ALL ERECTION BRACING REQUIRED TO KEEP THE STRUCTURE STABLE AND IN ALIGNMENT DURING CONSTRUCTION.
8. WHERE STRUCTURAL DRAWINGS INDICATE PROTECTED ZONES, DO NOT FASTEN OR OTHERWISE CONNECT ANY STRUCTURAL OR NON-STRUCTURAL ELEMENT TO THOSE ZONES, WITH EXCEPTION OF STEEL DECK CONNECTED WITH 19 (3/4") PUDDLE WELDS. FAILURE TO ADHERE WILL REQUIRE COMPLETE REPLACEMENT OF THE AFFECTED MEMBER.
9. WHERE MOMENT CONNECTIONS ARE SHOWN ON PLAN PROVIDE CONNECTION FOR FULL MEMBER CAPACITY UNLESS NOTED OTHERWISE.
10. DISTRIBUTE HANGER LOADS FROM MECHANICAL AND HEAVY ELECTRICAL SERVICES SUSPENDED FROM STEELWORK UNIFORMLY ALONG MEMBERS. DO NOT CAUSE TWISTING OF STEEL MEMBERS OR EXCESSIVE BENDING OF MEMBER FLANGES. ALTERNATE HANGER POSITION ON EITHER SIDE OF MEMBERS.
11. WHERE SEISMIC RESTRAINTS ARE REQUIRED FOR NON-STRUCTURAL ELEMENTS, APPLY LOADS IN THE PLANE OF PRIMARY STRUCTURAL ELEMENTS, AND DO NOT EXCEED 1.5kN.
12. DO NOT APPLY LATERAL LOADS TO MEMBERS UNLESS APPROVED BY THE CONSULTANT.
13. AESS MEMBERS: ERECT USING SOFTENED SLINGS OR OTHER METHODS SUCH THAT THEY ARE NOT DAMAGED. PROVIDE PADDING AS REQUIRED TO PROTECT WHILE RIGGING AND ALIGNING. WELD TABS FOR TEMPORARY BRACINGS AND SAFETY CABLING ONLY AT POINTS CONCEALED FROM VIEW IN THE COMPLETE STRUCTURE OR WHERE APPROVED BY THE CONSULTANT.
14. AESS MEMBERS: REMOVE ALL FIELD CONNECTION AIDS ADDED TO ALLOW ALIGNMENT, FIT UP AND WELDING. REMOVE WELDS AT RUN-OUT TABS TO MATCH ADJACENT SURFACE, AND GRIND SMOOTH. PLUG WELD AND GRIND SMOOTH HOLES FOR ERECTION BOLTS.
15. GROUT UNDER BASE PLATES AND BEARING PLATES USING PROCEDURES IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. PROVIDE 100% CONTACT OVER GROUTED AREA.
16. GROUT COLUMN BASE PLATES AND BEAM BEARING PLATES AS SOON AS STEELWORK IS COMPLETED. DO NOT ADD LOAD ON STEELWORK UNTIL GROUTING IS COMPLETED AND GROUT STRENGTH HAS REACHED AT LEAST 20 MPA.

LOAD BEARING MASONRY

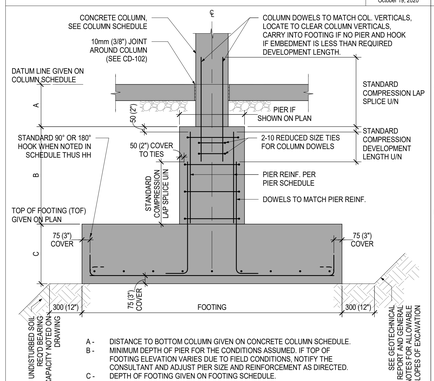
GN-017
December 22, 2021

1. CONFORM TO CSA A371 "MASONRY CONSTRUCTION FOR BUILDINGS" AND CSA S304.1 "DESIGN OF MASONRY STRUCTURE
2. MATERIALS:
   - A. HOLLOW BLOCK: CSA A165.1 - N15/A/M
   - B. SOLID BLOCK: CSA A165.1 - SF/15/A/M
   - C. MORTAR: CSA A179 - TYPE S
   - D. MASONRY GROUT: CSA A179M - COARSE GROUT, 1:3:2 - CEMENT: SAND: PEA-STONE BY VOLUME, WITH SLUMP, OR APPROVED PRE-BAGGED MATERIAL
3. MASONRY CONTRACTOR IS TO BE A MEMBER OF THE CANADIAN MASONRY CONTRACTORS ASSOCIATION.
4. NON LOAD-BEARING MASONRY WALLS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, SEE ARCHITECTURAL DRAWINGS.
5. LAY UNITS IN RUNNING BOND. ALL FACE SHELLS TO BE FULLY BEDDED, UNLESS NOTED OTHERWISE.
6. DO NOT USE MORTAR WHERE GROUT IS SPECIFIED.
7. PROVIDE LADDER TYPE HORIZONTAL JOINT REINFORCING PER ARCHITECTURAL SPECIFICATIONS, UNLESS OTHERWISE NOTED. USE CORNER TYPE LADDER REINFORCING AT ALL MASONRY WALL CORNERS AND INTERSECTIONS.
8. PROVIDE LATERAL SUPPORT AT TOPS OF ALL WALLS, REFER TO ARCHITECTURAL PROJECT DETAILS AND ARCHITECTURAL SPECIFICATIONS.
9. UNLESS OTHERWISE NOTED, PROVIDE MINIMUM 25 (1") DEFLECTION GAP AT TOP OF ALL NON LOAD BEARING MASONRY WALLS.
10. WHERE MASONRY WALLS ABUT COLUMNS OR CONCRETE WALLS, PROVIDE 25 WIDE MOVEMENT JOINTS BETWEEN THEM AND FILL WITH COMPRESSIBLE MATERIAL.
11. PROVIDE VERTICAL MOVEMENT JOINTS (MJ) IN LOAD BEARING WALLS AT LOCATIONS INDICATED ON PLANS, FILL WITH COMPRESSIBLE MATERIAL AS PER ARCHITECTURAL SPECIFICATIONS. EXTEND MOVEMENT JOINTS HEIGHT OF WALL, TERMINATING AT THE UNDERSIDE OF CONTINUOUS BOND BEAMS AT THE FLOOR OR ROOF (WHERE APPLICABLE).
12. PROVIDE VERTICAL MOVEMENT JOINTS (MJ) IN NON-LOAD BEARING WALLS AS INDICATED ON ARCHITECTURAL DRAWINGS AND SPECIFICATIONS.
13. PROVIDE STANDARD LINTELS OVER ALL OPENINGS IN NON-LOAD BEARING MASONRY WALLS AS SHOWN ON TYPICAL DETAILS. THE OPENINGS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.
14. PROVIDE MIN. 190 (7-1/2") DEEP BOND BEAMS AT TOP OF WALLS. REINFORCE WITH MIN. 1-15 TOP AND BOTTOM CONTINUOUS AND GROUT SOLID. CARRY ALL VERTICAL REINFORCING BARS CONTINUOUSLY THROUGH BOND BEAM.
15. REINFORCED MASONRY:
    - A. SEE PLANS AND DETAILS FOR STRUCTURAL MASONRY REINFORCING.
    - B. FOR NON-LOAD BEARING MASONRY, SEE TYPICAL DETAILS FOR MINIMUM SEISMIC MASONRY REINFORCING.
    - C. MASONRY CELLS TO BE REINFORCED ARE TO BE CLEAR OF CONDUITS AND OVER-HANGING MORTAR OR DEBRIS.
    - D. SPLICES FOR MASONRY WALL REINFORCING TO BE:

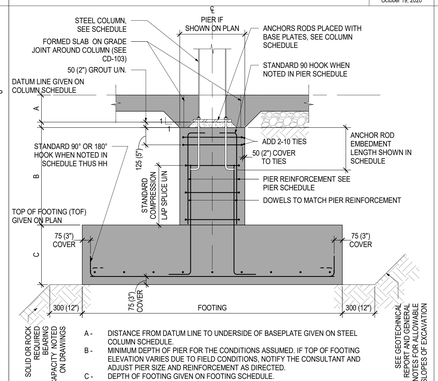
WIRE REINFORCING	200 (8")
10M BARS	450 (18")
15M BARS	650 (26")
20M BARS	900 (36")
25M BARS	1300 (51")

    - E. LOCATE VERTICAL BARS ACCURATELY WITHIN CELLS AS INDICATED ON DRAWINGS. HOLD IN POSITION AND BOTTOM. USE REBAR POSITIONERS AS REQUIRED.
    - F. PROVIDE VERTICAL DOWELS AT BASE OF WALLS TO MATCH VERTICAL REINFORCING. EMBED INTO SLAB WITH STANDARD HOOKS OR PROVIDE CLASS B LAP INTO FOUNDATION WALLS BELOW (AS APPLICABLE).
    - G. REINFORCE ALL SIDES OF WALL OPENINGS EXCEEDING 1000 (3'-6") IN WIDTH OR HEIGHT WITH ADDITIONAL 1-15 HORIZONTAL AND VERTICAL AS APPROPRIATE. EXTEND 600 (2'-0") BEYOND OPENING. PLACE HORIZONTAL BARS IN BLOCKS WITH PREFORMED CELLS WHICH ALLOW PASSING OF VERTICAL BARS. WHERE STEEL LINTELS ARE USED, OFFSET VERTICALS TO CLEAR LINTEL BEARING.
    - H. ADD 1-15 VERTICAL AT WALL ENDS AND ON EACH SIDE OF MOVEMENT JOINTS.
16. GROUTING MASONRY:
    - A. GROUT CELLS CONTAINING VERTICAL OR HORIZONTAL REINFORCEMENT ONLY. IF MASONRY WALLS ARE AS "GROUTED SOLID" ON PLAN, THEN GROUT ALL CELLS FULLY.
    - B. USE LOW LIFT GROUTING PROCEDURE UNLESS OTHERWISE APPROVED IN WRITING BY THE STRUCTURAL CONSULTANT. PLACE GROUT IN LIFTS NOT EXCEEDING 1500 (5'-0") HEIGHT. TERMINATE EACH LIFT 40 (1 ½") BELOW TOP OF MASONRY UNIT.
    - C. UNLESS OTHERWISE NOTED, PREFILL VOIDS IN MASONRY UNITS BELOW BEAM AND JOIST SUPPORTS WITH GROUT FOR A MINIMUM VERTICAL DEPTH OF 600 (2'-0") AND A LENGTH EXTENDING MINIMUM 200 (8") BEYOND THE EDGES OF BEARING PLATES. SEE TYPICAL DETAILS FOR ADDITIONAL REINFORCING AND GROUTING UNDER ROOF BEAMS AND JOISTS.
    - D. FULLY GROUT ALL WALL POCKETS AFTER INSTALLATION OF STEEL

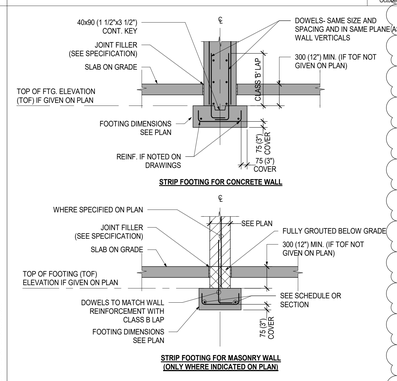
FOOTING AT CONCRETE COLUMN — CD-001
October 19, 2020



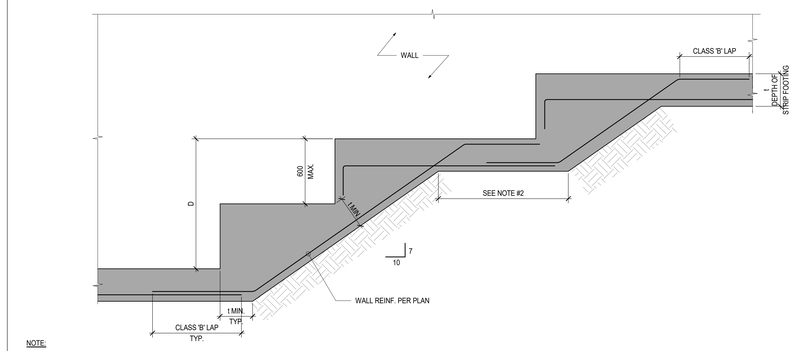
FOOTING AT STEEL COLUMN — CD-002
October 19, 2020



STRIP FOOTING FOR WALLS — CD-007
October 19, 2020

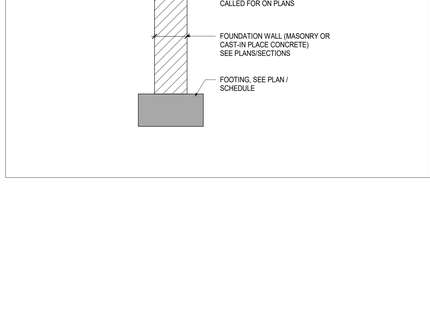


STEPPING OF STRIP FOOTING — CD-008
July 22, 2026

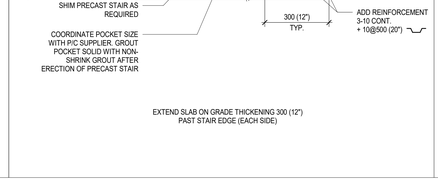


NOTE:
1. SLOPE NOT TO EXCEED 7V:10H UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT.
2. IF TOTAL STEPPING "D" EXCEED 1200mm PROVIDE INTERMEDIATE HORIZONTAL SECTION OF A MINIMUM LENGTH OF 1200mm.

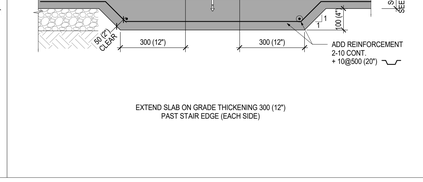
SUMP PIT DETAIL FOR PITS OVER 1800(6'-0") DEEP — CD-010
March 17, 2020



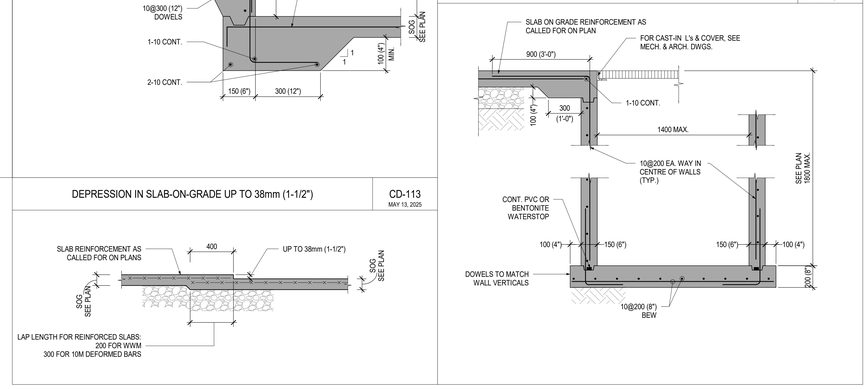
BACKFILL AROUND PIPE UNDER STRIP FOOTING — CD-011
October 19, 2020



SLOPE BETWEEN ADJACENT EXCAVATIONS — CD-012
October 19, 2020



SLAB ON GRADE THICKENING UNDER CONCRETE STAIRS — CD-106
October 19, 2020



TYPICAL SLAB ON GRADE JOINTS AROUND STEEL COLUMN — CD-103
March 17, 2020

SAWCUT CONTROL JOINT IN SLAB ON GRADE — CD-100
October 19, 2020

CONSTRUCTION JOINT IN SLAB ON GRADE — CD-101
October 19, 2020

SLAB ON GRADE JOINTS AROUND CONCRETE COLUMN — CD-102
March 17, 2020

SLAB ON GRADE THICKENING UNDER MASONRY PARTITIONS — CD-105
October 19, 2020

TRENCH DRAIN IN SLAB ON GRADE — CD-109
October 19, 2020

CHANGE IN SLAB ON GRADE LEVEL OF UP TO 200 (8") — CD-111
March 17, 2020

SLAB ON GRADE CONTROL JOINT AT INTERIOR DOORWAY — CD-104
March 17, 2020

SLAB ON GRADE THICKENING UNDER PRECAST STAIRS — CD-107
October 19, 2020

SLAB ON GRADE THICKENING UNDER STEEL STAIRS — CD-108
October 19, 2020

CHANGE IN SLAB ON GRADE LEVEL OF 200 (8") UP TO 600 (24") — CD-110
October 19, 2020

STAIR ON SLAB ON GRADE — CD-112
October 19, 2020

DEPRESSION IN SLAB-ON-GRADE UP TO 38mm (1-1/2") — CD-113
MAY 13, 2025

SUMP PIT DETAIL FOR PITS MAX. 1800 (6'-0") DEEP — CD-009
March 17, 2020

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

If this sheet is not 46 7/8" x 33 1/8" (1189 x 841 mm) It is a reduced print - Read dwg. accordingly.

Contractors must check and verify all dimensions on the job and report any discrepancies to the Architect before proceeding with the work.

All prints and specifications are the property of the Architect and must be returned at the completion of the work.

Drawings should not be scaled.

No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER
6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

consultants

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civil engineer — SCS Consulting Group Ltd., 30 Centurian Drive, Suite 100, Markham, ON, L3R 8B8, Phone.905.475.1900, Fax.905.475.8335

structural engineer — LEA Consulting Ltd., 40 University Ave, Suite 503, Toronto, ON, M5J 1T1, Phone.905.470.0015

mechanical engineer — Smith + Andersen, 4211 Yonge Street, Toronto, ON, M2P 2A9, Phone.416.487.8151

electrical engineer — Smith + Andersen, 4211 Yonge Street, Toronto, ON, M2P 2A9, Phone.416.487.8151

landscape architect — Quinn Design Associates Inc., 180 John Street, Toronto, ON, M5T 1X5, Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER — 2026-07-22 — E.JI.CHOWDHURY — 100072722 — PROVINCE OF ONTARIO

SHS LEA — LEA Consulting Ltd. 40 University Avenue, Suite 503, Toronto, Ontario M5J 1T1 — Tel. 905 470 0015, Fax. 905 470 0030, WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

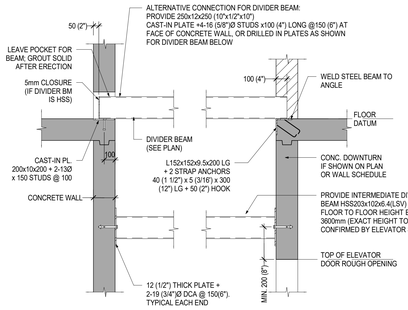
TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S102
	Revision 6

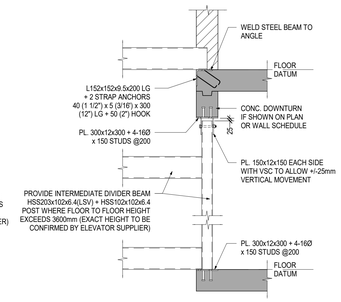
A0 FILENAME: Autodesk Docs://2302 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-22 3:08:44 PM

ELEVATOR DIVIDER BEAM — GD-001 — October 29, 2020

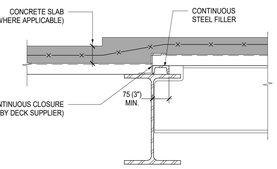


CONDITION A

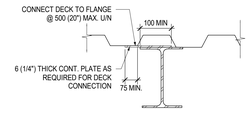


CONDITION B

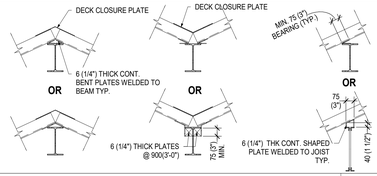
STEEL DECK AT CHANGE IN LEVEL — SD-201 — March 17, 2020



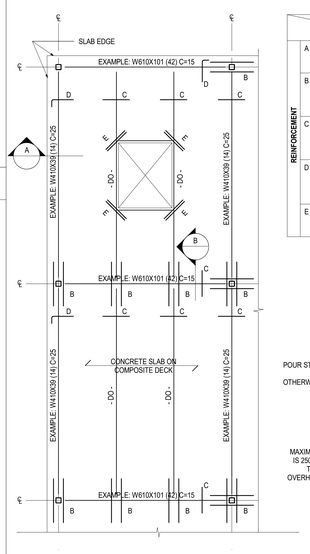
STEEL BEAM PARALLEL OR ANGLED TO STEEL DECK — SD-203 — October 29, 2020



SUPPORT OF SLOPED ROOF DECK — SD-202 — October 29, 2020



COMPOSITE STEEL BEAM CONSTRUCTION — SD-300 — October 29, 2020

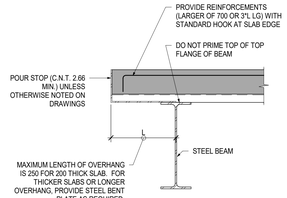


FRAMING PLAN

			SLAB THICKNESS (t)	
			LESS THAN 89	89 OR MORE
REINFORCEMENT	A	MINIMUM SLAB REINFORCEMENT	WWF 152x152 MW18.7/MW18.7 TOP	10@400 TOP EA.WAY
	B	ADDED LONGITUDINAL REINFORCEMENT AT ENDS OF COMPOSITE BEAMS	2-15T x 2000 LG.	2-15T x 2000 LG.
	C	ADDED TRANSVERSE REINFORCEMENT OVER COMPOSITE BEAMS	10@500T x1200 LG.	NOT REQUIRED
	D	ADDED TRANSVERSE REINFORCEMENT OVER COMPOSITE BEAMS	15@500 T REFER TO SECTION 'A'	NOT REQUIRED
	E	ADDED REINFORCEMENT AT OPENINGS	2-10x1200 LG. IN MIDDLE OF SLAB	2-10x1200 LG. IN MIDDLE OF SLAB

NOTES:

1. COMPOSITE MEMBERS ARE IDENTIFIED BY "(XX)" FOLLOWING THE STEEL SECTION SIZE. WHERE "(XX)" DENOTES NUMBER OF STUDS REQUIRED FOR MEMBER.
2. "C=YY" IS THE AMOUNT OF POSITIVE CAMBER AT THE MIDSPAN OF THE COMPOSITE BEAM.
3. STUD DIAMETER IS 19 (3/4") UNLESS OTHERWISE SPECIFIED.
4. STUD LENGTH AFTER WELDING SHALL BE THE OVERALL DEPTH OF STEEL DECK PLUS SLAB THICKNESS MINUS 25 (1").
5. SPACING OF STUDS SHALL NOT EXCEED 600 (2'-0") INCREASE THE NUMBER OF STUDS SHOWN ON PLANS AS REQUIRED TO MEET THIS CRITERION.
6. DESIGN CONNECTIONS FOR COMPOSITE BEAMS FOR ONE AND A HALF TIMES THE END REACTION DUE TO THE UDL CAPACITY OF THE STEEL SECTION UNLESS A LARGER REACTION IS GIVEN ON PLAN.
7. LAP WELDED WIRE FABRIC 200 (8"). FOR CONTINUOUS DEFORMED BARS PROVIDE CLASS 'B' LAP AND HOOK AT PERIMETER. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN WIRE FABRIC OR REBARS 25 (1") BELOW TOP OF SLAB.



SECTION 'A'

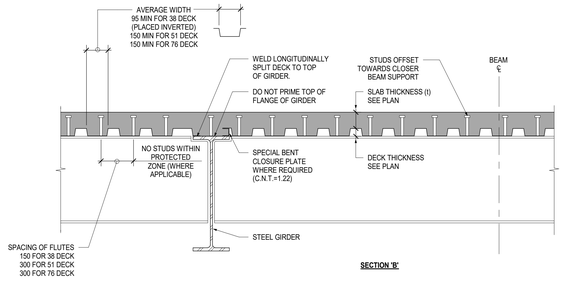
USE THE FOLLOWING CRITERIA TO LAY OUT STUDS:

1. DO NOT WELD ANY STUDS TO BEAMS OR GIRDERS UNTIL ALL STUDS FOR THAT MEMBER HAVE BEEN LAID OUT ON SITE.
2. DO NOT PLACE STUDS ON THE CANTILEVERED PORTION OF A COMPOSITE BEAM UNLESS OTHERWISE NOTED ON THE DRAWINGS.
3. STEEL GIRDERS: LAY OUT SINGLE ROW OF STUDS ON CENTRELINE OF BEAM UNIFORMLY SPACED AT MINIMUM 150 (6") AND MAXIMUM 600 (24"). IF THE NUMBER OF STUDS EXCEEDS THE NUMBER THAT CAN BE ACCOMMODATED WITH MINIMUM SPACING IN ONE ROW, LAY OUT BALANCE IN DOUBLE ROW AS SHOWN IN THE PLAN "D" BELOW.
4. STEEL BEAM: THE STUD CANNOT BE PLACED IN CENTRE OF FLUTE, OFFSET THE STUD TOWARDS THE CLOSER BEAM SUPPORT. THE MINIMUM DISTANCE REQUIRED TO MISS UPSTAND IN FLUTE. SEE SECTION 'B' BELOW

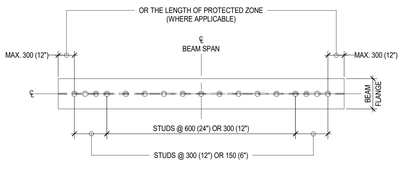
STEP 1: LAY OUT SINGLE ROW OF STUDS ON CENTERLINE OF BEAM UNIFORMLY SPACED @ 600 (2'-0").

STEP 2: LAY OUT THE REMAINING STUDS HALF WAY BETWEEN THE ALREADY PLACED STUDS, STARTING FROM EACH END. REPEAT THE PROCESS AS REQUIRED. SEE PLAN "C" BELOW.

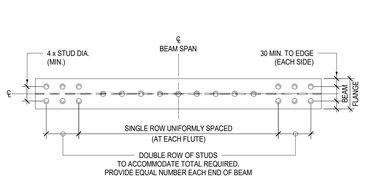
STEP 3: IF THE NUMBER OF STUDS EXCEEDS THE NUMBER THAT CAN BE ACCOMMODATED BY FILLING EACH FLUTE IN ONE ROW, LAY OUT BALANCE IN DOUBLE ROW FROM BOTH ENDS AS SHOWN IN THE PLAN "D" BELOW.



SECTION 'B'



PLAN 'C'



PLAN 'D'

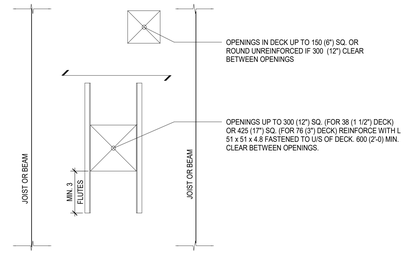
REINFORCEMENT FOR SLAB ON COMPOSITE DECK — SD-204 — October 29, 2020

SLAB THICKNESS (t)	LESS THAN 89 (3 1/2")	89 (3 1/2") OR MORE
REINFORCEMENT	WWF 152x152 MW18.7/MW18.7	10@400 (16") E.W.

NOTES:

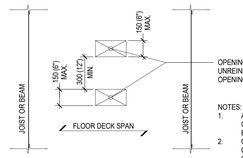
1. LAP WELDED WIRE FABRIC 200 (8").
2. PROVIDE "CLASS "B" LAP FOR ALL REBARS.
3. PROVIDE STANDARD HOOKS ALONG SLAB EDGES.
4. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN WIRE FABRIC OR REBARS 25 (1") BELOW TOP OF SLAB.
5. FOR ADDITIONAL REINFORCEMENT AT COMPOSITE BEAMS, SEE DETAIL FOR TYPICAL COMPOSITE STEEL BEAM CONSTRUCTION.

ROOF DECK OPENINGS — SD-206 — March 17, 2020



- ALL LARGER OPENINGS AND OPENINGS SPACED CLOSER THAN SPECIFIED ABOVE TO BE FRAMED BY STRUCTURAL STEEL, SEE DRAWING NOTES.

FLOOR DECK OPENINGS — SD-207 — October 29, 2020

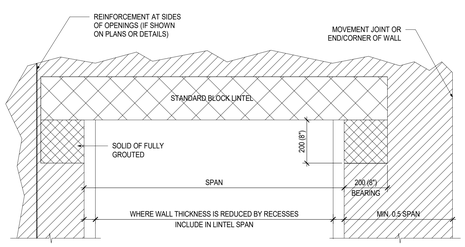


OPENINGS IN DECK UP TO 150 (6") WIDE UNREINFORCED IF 300 (12") CLEAR BETWEEN OPENINGS

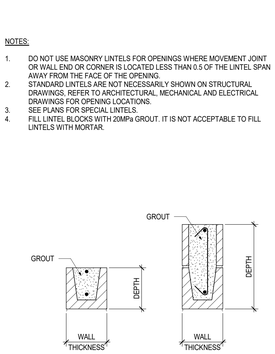
NOTES:
1. ALL LARGER OPENINGS AND OPENINGS SPACED CLOSER THAN SPECIFIED IN DIAGRAM TO BE FRAMED BY STRUCTURAL STEEL, SEE DRAWING NOTES.
2. CUT STEEL DECK FOR THE OPENINGS AFTER CONCRETE REACHES MINIMUM 75% OF SPECIFIED CONCRETE STRENGTH

TYPICAL MASONRY LINTELS IN NON-LOAD BEARING WALLS — MD-300 — October 29, 2020

SEE SD-312 FOR STANDARD STEEL LINTELS



WALL THICKNESS	SPAN	BLOCK LINTEL DEPTH	REINFORCEMENT TOP & BOTTOM
90 (4")	UP TO 1200 (4'-0")	190	1-10
	1200 (4'-0") TO 1800 (6'-0")	N/A	N/A
	1800 (6'-0") TO 2400 (8'-0")	N/A	N/A
	2400 (8'-0") TO 3000 (10'-0")	N/A	N/A
140 (6")	UP TO 1200 (4'-0")	190	1-10
	1200 (4'-0") TO 1800 (6'-0")	190	1-10
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	N/A	N/A
190 (8")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP
240 (10")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP
290 (12")	UP TO 1200 (4'-0")	190	1-15
	1200 (4'-0") TO 1800 (6'-0")	190	1-15
	1800 (6'-0") TO 2400 (8'-0")	390	1-10 +1-10@200 (8") STIRRUP
	2400 (8'-0") TO 3000 (10'-0")	390	1-10 +1-10@200 (8") STIRRUP



NOTES:

1. DO NOT USE MASONRY LINTELS FOR OPENINGS WHERE MOVEMENT JOINT OR WALL END OR CORNER IS LOCATED LESS THAN 0.5 OF THE LINTEL SPAN AWAY FROM THE FACE OF THE OPENING.
2. STANDARD LINTELS ARE NOT NECESSARILY SHOWN ON STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENING LOCATIONS.
3. SEE PLANS FOR SPECIAL LINTELS.
4. FILL LINTEL BLOCKS WITH 20MPa GROUT. IT IS NOT ACCEPTABLE TO FILL LINTELS WITH MORTAR.

MASONRY LINTEL

Key to Detail Location

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2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER
6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

TYPICAL DETAILS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S108
	Revision: 6

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

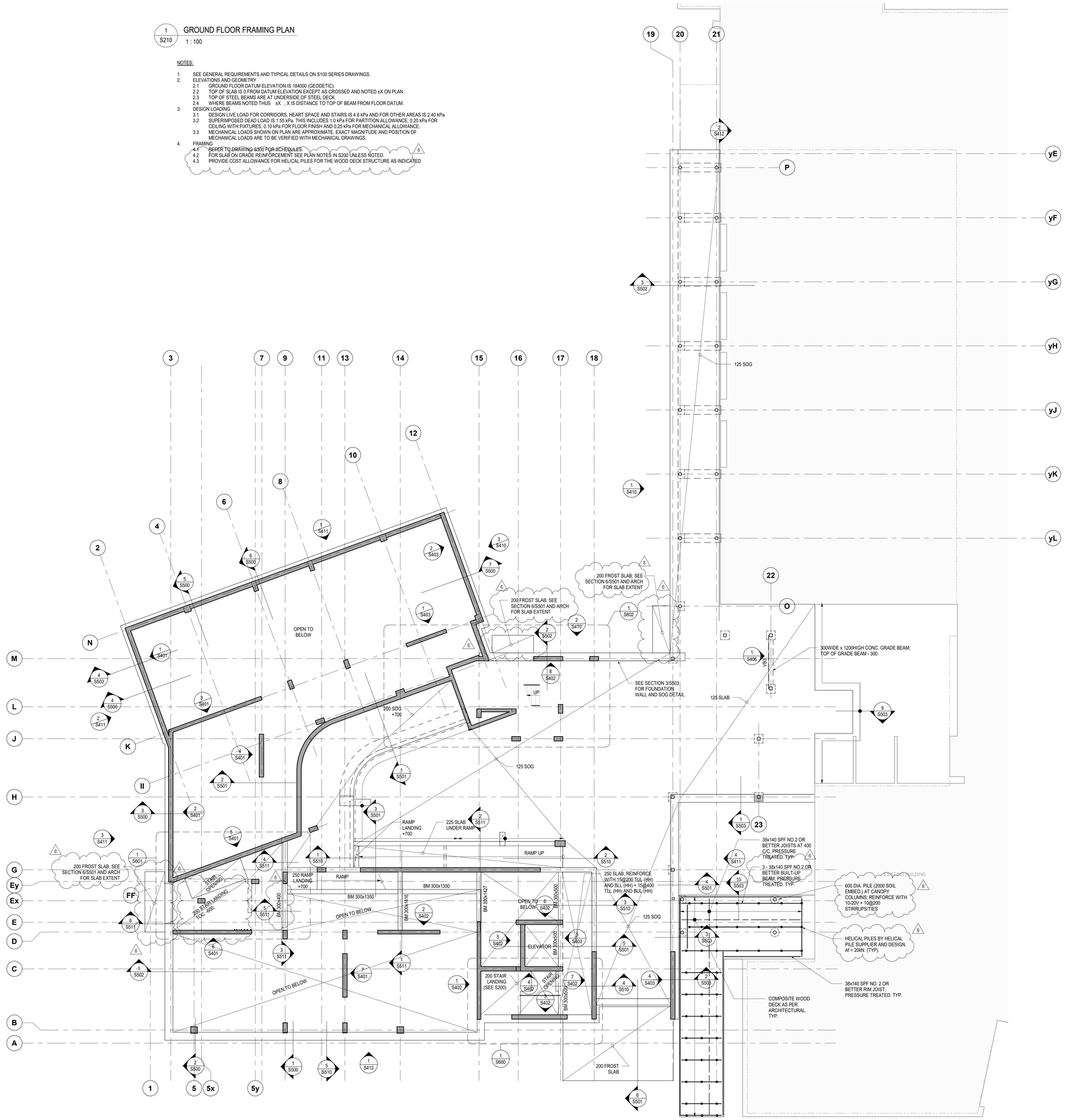
DRAWING LAST EDITED: 2026-07-22 3:08:45 PM

1 S210 GROUND FLOOR FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 GROUND FLOOR DATUM ELEVATION IS 184000 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 WHERE BEAMS NOTED THUS ±X , X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S500 FOR SCHEDULES.
   - 4.2 FOR SLAB ON GRADE REINFORCEMENT SEE PLAN NOTES IN S200 UNLESS NOTED.
   - 4.3 PROVIDE COST ALLOWANCE FOR HELICAL PILES FOR THE WOOD DECK STRUCTURE AS INDICATED



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5	2026-07-08	ISSUED FOR TENDER
6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

GROUND FLOOR FRAMING PLAN

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S210
	Revision: 6

A0 FILENAME: Autodesk Docs://1032 Sterling Hall/22317_SHS-STR-R25.rvt

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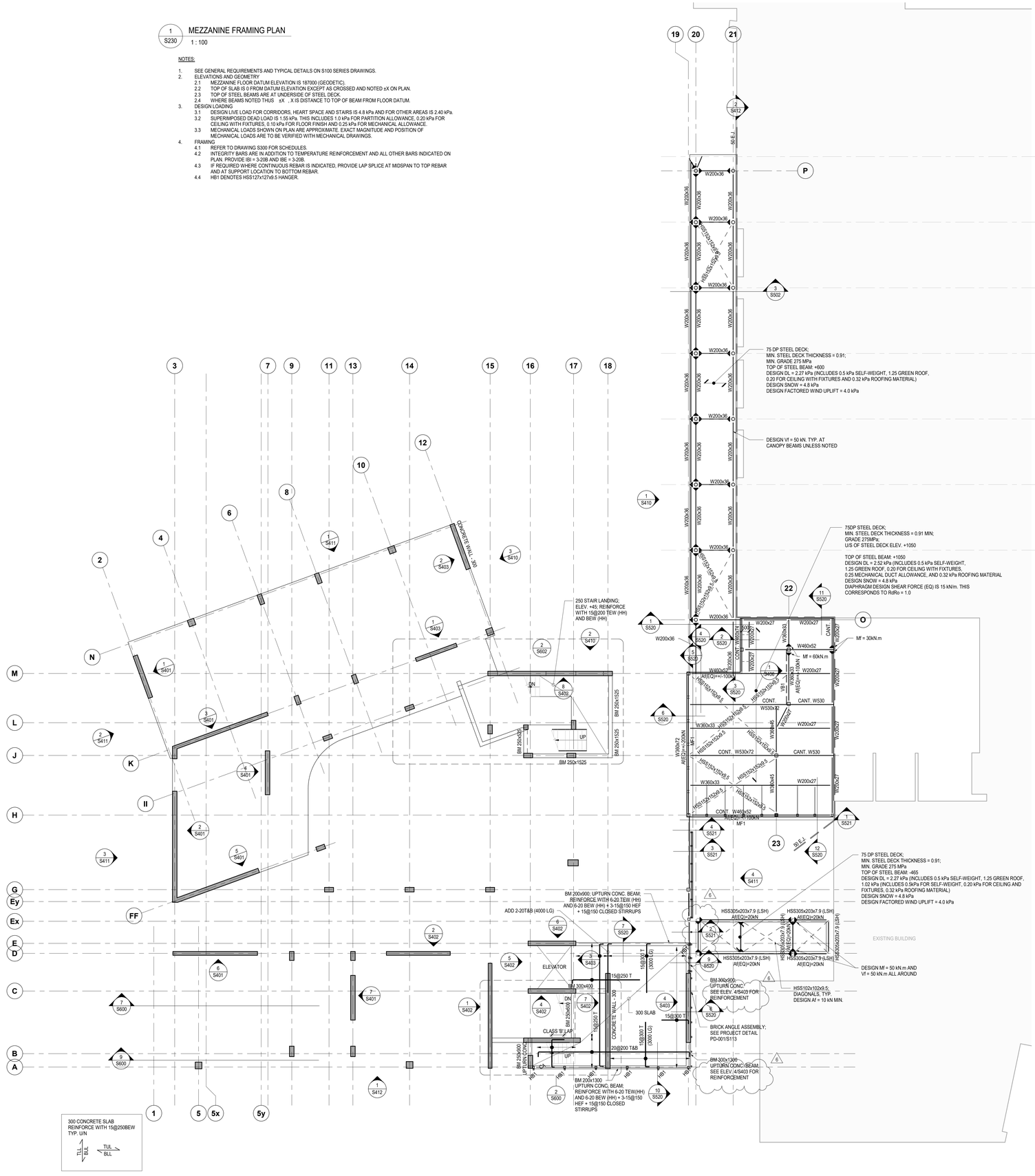
1 S230 MEZZANINE FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 MEZZANINE FLOOR DATUM ELEVATION IS 187000 (GEODETIC).
   - 2.2 TOP OF SLAB IS 0 FROM DATUM ELEVATION EXCEPT AS CROSSED AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 WHERE BEAMS NOTED THUS ±X , X IS DISTANCE TO TOP OF BEAM FROM FLOOR DATUM.
3. DESIGN LOADING
   - 3.1 DESIGN LIVE LOAD FOR CORRIDORS, HEART SPACE AND STAIRS IS 4.8 kPa AND FOR OTHER AREAS IS 2.40 kPa.
   - 3.2 SUPERIMPOSED DEAD LOAD IS 1.55 kPa. THIS INCLUDES 1.0 kPa FOR PARTITION ALLOWANCE, 0.20 kPa FOR CEILING WITH FIXTURES, 0.10 kPa FOR FLOOR FINISH AND 0.25 kPa FOR MECHANICAL ALLOWANCE.
   - 3.3 MECHANICAL LOADS SHOWN ON PLAN ARE APPROXIMATE. EXACT MAGNITUDE AND POSITION OF MECHANICAL LOADS ARE TO BE VERIFIED WITH MECHANICAL DRAWINGS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES.
   - 4.2 INTEGRITY BARS ARE IN ADDITION TO TEMPERATURE REINFORCEMENT AND ALL OTHER BARS INDICATED ON PLAN. PROVIDE IBI = 3-20B AND IBE = 3-20B.
   - 4.3 IF REQUIRED WHERE CONTINUOUS REBAR IS INDICATED, PROVIDE LAP SPLICE AT MIDSPAN TO TOP REBAR AND AT SUPPORT LOCATION TO BOTTOM REBAR.
   - 4.4 HB1 DENOTES HSS127x127x9.5 HANGER.



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MEZZANINE LEVEL FRAMING PLAN

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S230
	Revision: 6

A0

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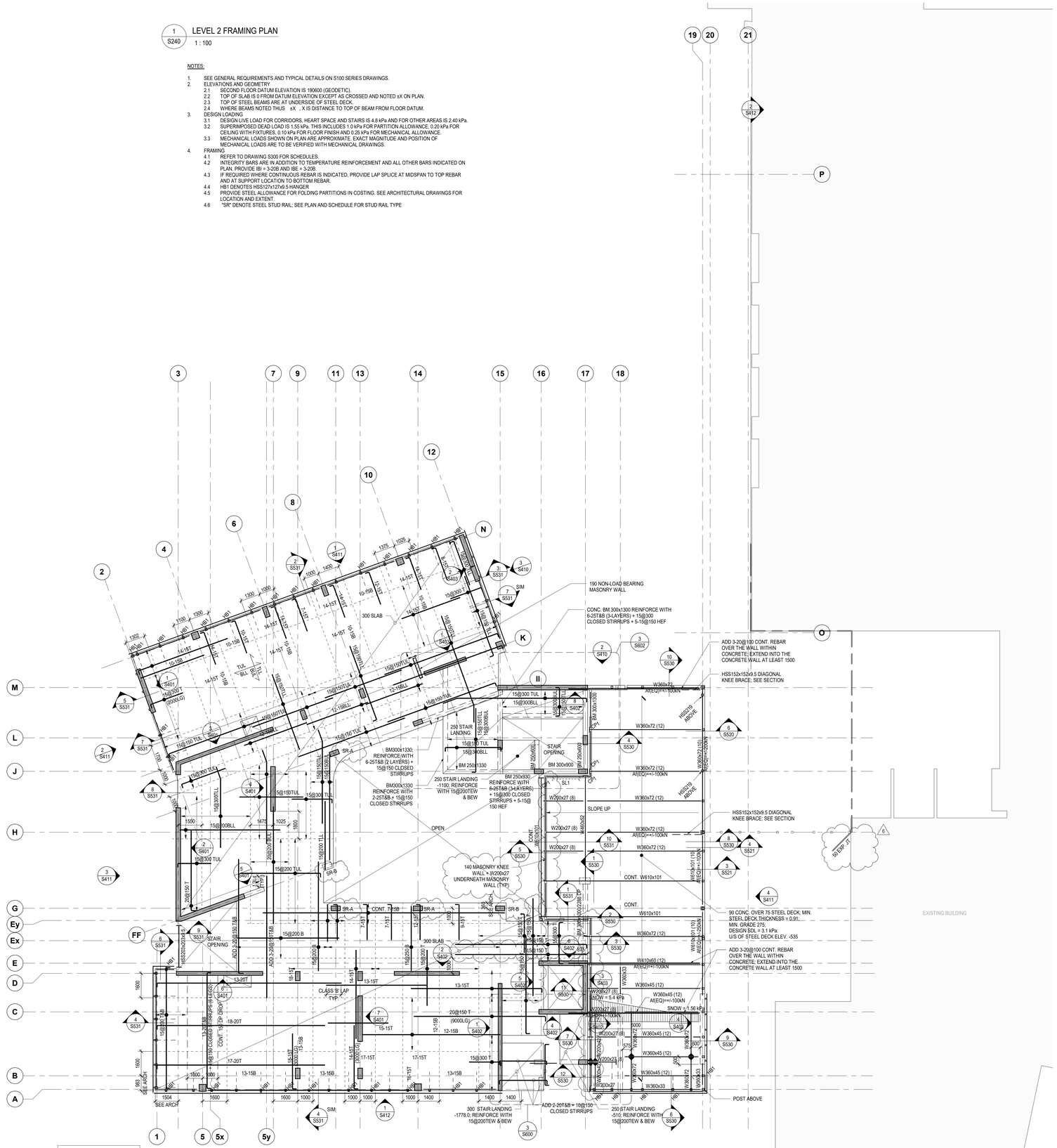

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6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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PROJECT NORTH

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SECOND FLOOR FRAMING PLAN

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S240
Revision:	6

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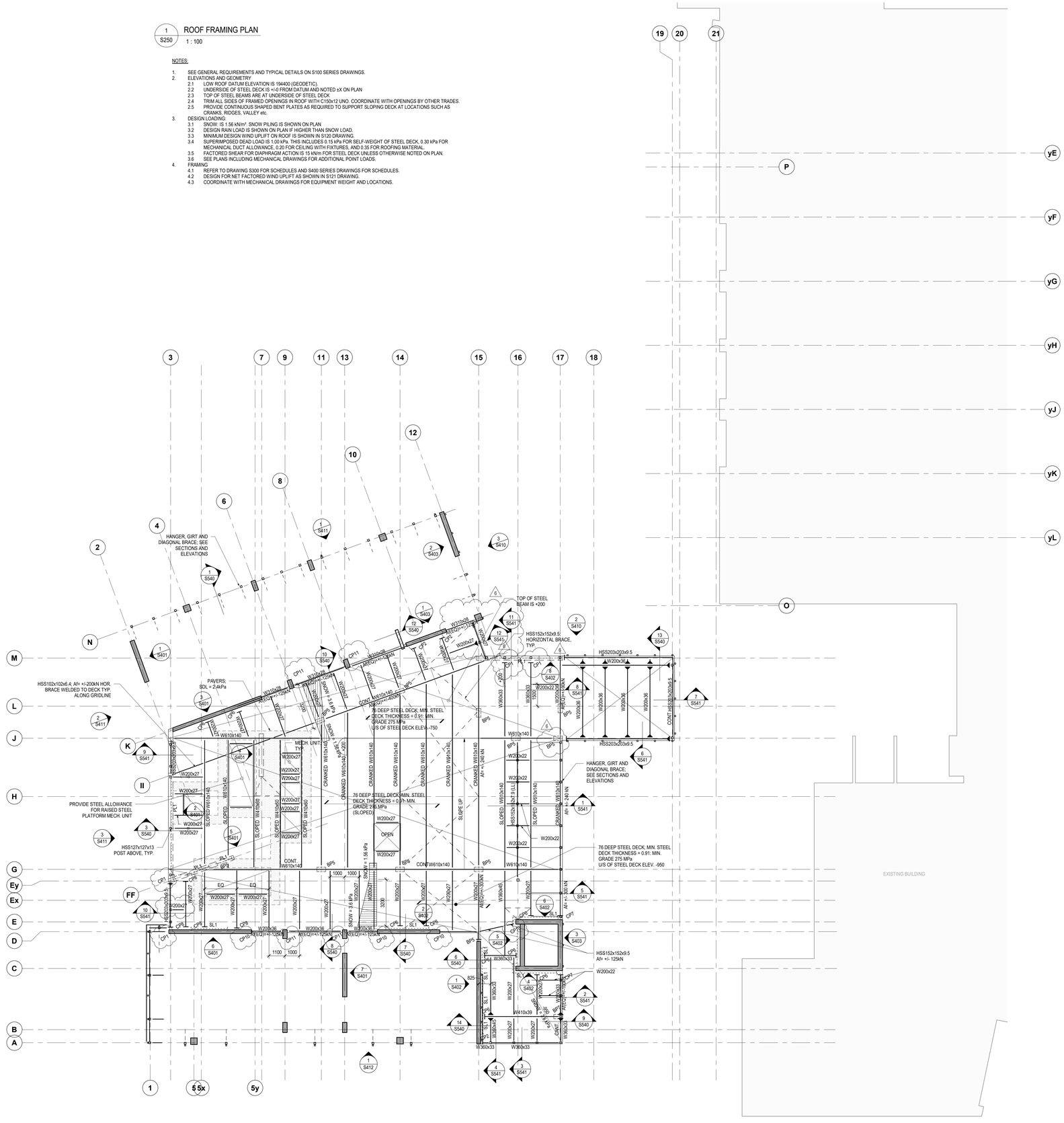
1 / S250 ROOF FRAMING PLAN

1 : 100

NOTES:

1. SEE GENERAL REQUIREMENTS AND TYPICAL DETAILS ON S100 SERIES DRAWINGS.
2. ELEVATIONS AND GEOMETRY
   - 2.1 LOW ROOF DATUM ELEVATION IS 194400 (GEODETIC).
   - 2.2 UNDERSIDE OF STEEL DECK IS +/-0 FROM DATUM AND NOTED ±X ON PLAN.
   - 2.3 TOP OF STEEL BEAMS ARE AT UNDERSIDE OF STEEL DECK.
   - 2.4 TRIM ALL SIDES OF FRAMED OPENINGS IN ROOF WITH C150x12 UNO. COORDINATE WITH OPENINGS BY OTHER TRADES.
   - 2.5 PROVIDE CONTINUOUS SHAPED BENT PLATES AS REQUIRED TO SUPPORT SLOPING DECK AT LOCATIONS SUCH AS CRANKS, RIDGES, VALLEY etc.
3. DESIGN LOADING:
   - 3.1 SNOW: IS 1.56 kN/m². SNOW PILING IS SHOWN ON PLAN
   - 3.2 DESIGN RAIN LOAD IS SHOWN ON PLAN IF HIGHER THAN SNOW LOAD.
   - 3.3 MINIMUM DESIGN WIND UPLIFT ON ROOF IS SHOWN IN S120 DRAWING.
   - 3.4 SUPERIMPOSED DEAD LOAD IS 1.00 kPa. THIS INCLUDES 0.15 kPa FOR SELF-WEIGHT OF STEEL DECK, 0.30 kPa FOR MECHANICAL DUCT ALLOWANCE, 0.20 FOR CEILING WITH FIXTURES, AND 0.35 FOR ROOFING MATERIAL.
   - 3.5 FACTORED SHEAR FOR DIAPHRAGM ACTION IS 15 kN/m FOR STEEL DECK UNLESS OTHERWISE NOTED ON PLAN.
   - 3.6 SEE PLANS INCLUDING MECHANICAL DRAWINGS FOR ADDITIONAL POINT LOADS.
4. FRAMING
   - 4.1 REFER TO DRAWING S300 FOR SCHEDULES AND S400 SERIES DRAWINGS FOR SCHEDULES.
   - 4.2 DESIGN FOR NET FACTORED WIND UPLIFT AS SHOWN IN S121 DRAWING.
   - 4.3 COORDINATE WITH MECHANICAL DRAWINGS FOR EQUIPMENT WEIGHT AND LOCATIONS.



Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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No	Date	Revision/Issued
1	2025-12-15	ISSUED FOR 30% CONSTRUCTION DOCUMENTS
2	2026-03-13	ISSUED FOR BUILDING PERMIT
3	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
4	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
5	2026-07-08	ISSUED FOR TENDER
6	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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PROJECT NORTH

REGISTERED PROFESSIONAL ENGINEER — E.H. CHOWDHURY — 2026-07-22 — PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

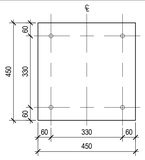
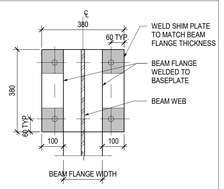
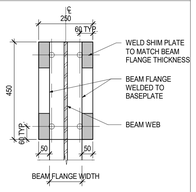
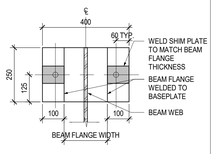
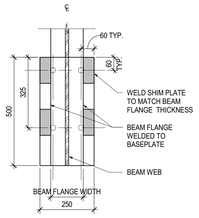
LOW ROOF FRAMING PLAN

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S250
	Revision: 6

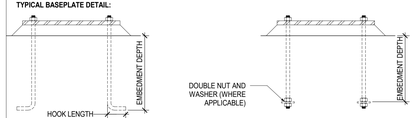
A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-22 3:08:49 PM

BASEPLATE SCHEDULE

MARK	SIZE	ANCHORS	DETAILS
BP1	450x25x450	4-19Ø ANCHOR RODS x 600 LG. EMBED + 75 HOOK AT EMBEDDED END	
BP2	450x30x450	4-25Ø ANCHOR RODS x 1000 LG. EMBED + DOUBLE NUT WITH WASHER AT EMBEDDED END	SEE BP1 DETAIL
BP3	380x20x380	4-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE
BP4	NOT USED	NOT USED	
BP5	450x20x250	4-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE 2. PROVIDE WALL POCKET
BP6	250x20x400	2-19DIAM. ANCHOR RODS x600LG + 75HOOK	 NOTE: 1. PROVIDE 50 GROUT BELOW BASEPLATE 2. PROVIDE WALL POCKET 3. CUT HOLES THRU FLANGES AS REQUIRED FOR ANCHOR INSTALLATION
BP7	250x30x500	4-25DIA. ANCHOR RODS x 1000 LG. EMBED. 75	

TYPICAL BASEPLATE DETAIL:

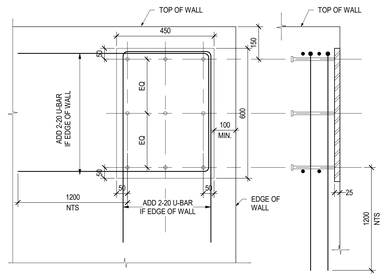
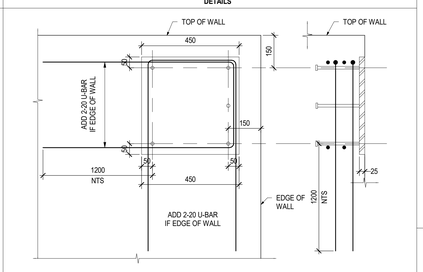
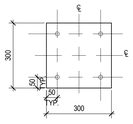
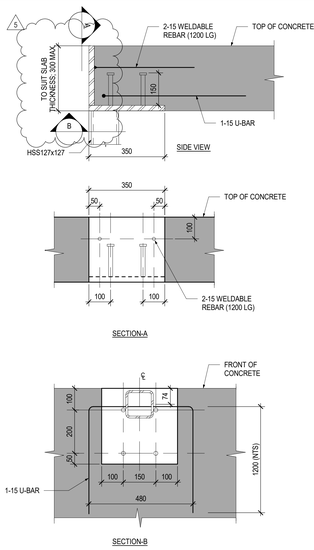
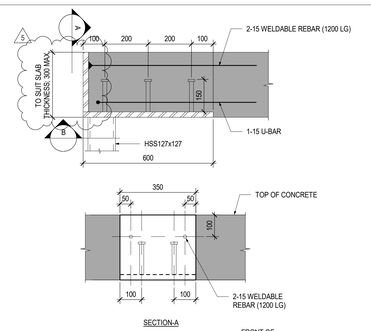
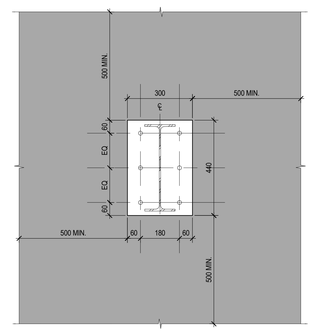
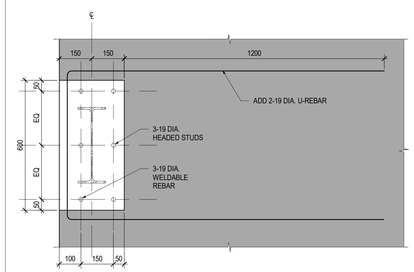
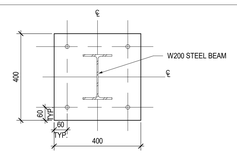


NOTE: PROVIDE ADDITIONAL COSTING ALLOWANCE OF 15% OF THE STRUCTURAL STEEL TONNAGE WEIGHT FOR STRUCTURAL STEEL CONNECTION (INCLUDING BASE PLATE AND CONNECTION PLATES). NOT ALL CONNECTION PLATES AND BASEPLATES ARE SHOWN IN THE SCHEDULE.

SHELF ANGLE AND PLATE SCHEDULE

MARK	DESCRIPTION	ANCHORS	REMARKS
SL1	CONT. L152x89x9.5 (LLV)	19Ø HILTI KWIK BOLT TZ2 @ 300 (121 EFFECTIVE EMBEDMENT)	
PL1	CONT. PL150x16	19Ø ANCHOR RODS WELDED @ 300 (1000 LG EMBEDMENT) WITH 75 HOOK	

CAST-IN PLATE SCHEDULE

MARK	SIZE	ANCHORS	DETAILS
CP1	600x25x450	6-19Ø x 200 LG. HEADED STUDS	 NOTE: BEAMS TO BE CENTERED TO THE PLATE HORIZONTALLY
CP2	450x25x450	6-19Ø x 200 LG. HEADED STUDS	**DETAILS**  NOTE: BEAMS TO BE CENTERED TO THE PLATE HORIZONTALLY
CP3	300x25x300	4-19 DIA x 200 LG. HEADED STUDS	 NOTE: BEAMS TO BE CENTERED TO THE PLATE
CP4	25 THICK CONNECTION PLATE SEE SECTION 2/S531	4-19 DIA x 200 LG. HEADED STUDS	
CP5	25 THICK CONNECTION PLATE SEE SECTION 6/S531	4-19 DIA x 200 LG. HEADED STUDS	
CP6	300x25x440	6-19 DIA. x 200 LG. HEADED STUDS	
CP7	300x25x600	3-19 DIA. WELDABLE REBAR (1000 LG) 3-19 DIA. x 200 LG. HEADED STUDS	
CP8	300x25x600	6-19 DIA. x 200 LG. HEADED STUDS	SEE DETAIL CP6
CP9	400x20x400	4-19 DIA. HILTI KWIK BOLT TZ2: 125 EFFECTIVE EMBEDMENT	

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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No	Date	Revision/Issued
1	2026-03-13	ISSUED FOR BUILDING PERMIT
2	2026-04-10	ISSUED FOR 75% CONSTRUCTION DOCUMENTS
3	2026-05-15	ISSUED FOR 90% CONSTRUCTION DOCUMENTS
4	2026-07-08	ISSUED FOR TENDER
5	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

consultants

architect — CS&P Architects Inc., 2345 Yonge St., Suite 200, Toronto, ON, M4P 2E5, Phone.416.482.5002, Fax.416.482.5040

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structural engineer — LEA Consulting Ltd., 40 University Ave, Suite 503, Toronto, ON, M5J 1T1, Phone.905.470.0015

mechanical engineer — Smith + Andersen, 4211 Yonge Street, Toronto, ON, M2P 2A9, Phone.416.487.8151

electrical engineer — Smith + Andersen, 4211 Yonge Street, Toronto, ON, M2P 2A9, Phone.416.487.8151

landscape architect — Quinn Design Associates Inc., 180 John Street, Toronto, ON, M5T 1X5, Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER — 2026-07-22 — E.V.L. HOBKIRK — 100072522 — PROVINCE OF ONTARIO

SHS LEA — LEA Consulting Ltd. 40 University Avenue, Suite 503, Toronto, Ontario M5J 1T1 — Tel: 905 470 0015, Fax: 905 470 0030, WWW.LEA.CA

THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

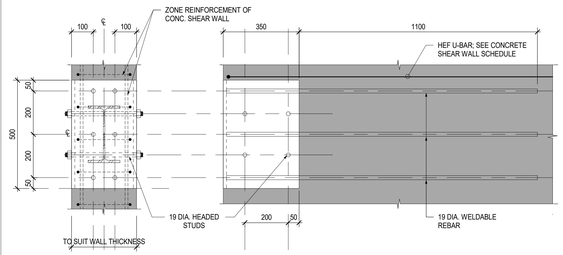
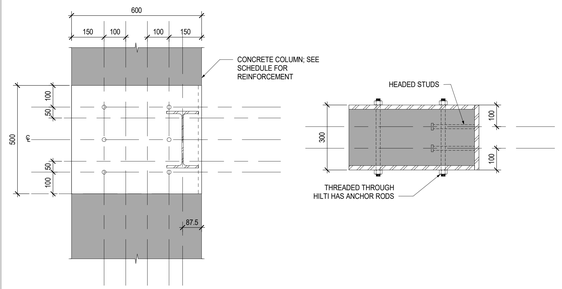
SCHEDULES

scale:	1 : 10
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S310
	Revision: 5

A0 FILENAME: Autodesk Docs://2102 Sterling Hall/22317_SHS-STR-R25.rvt

DRAWING LAST EDITED: 2026-07-22 3:08:50 PM

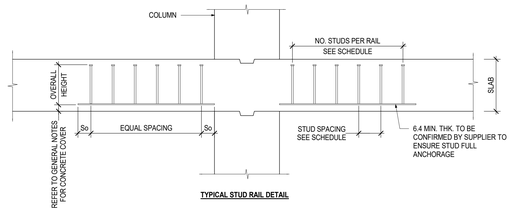
CAST-IN PLATE SCHEDULE

MARK	SIZE	ANCHORS	DETAILS
CP10	20 THICK WELDED U-PLATE	6- 19 DIA. WELDABLE REBAR + 4-19 DIA. THREADED HILTI HAS (ASTM F1554, GRADE 55) RODS	
CP11	20 THICK WELDED U-PLATE	6-19 DIA. THREADED HILTI HAS (ASTM F1554, GRADE 55) RODS + 4-19 DIA. HEADED STUDS (200 EMBED)	

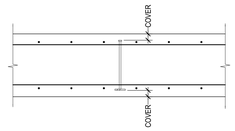
STUD RAIL SCHEDULE

MARK	NO. OF RAILS	STUD DIAMETER (mm)	OAH (mm)	STUD PER RAIL	So (mm)	SPACING (mm)	LENGTH (mm)
SR-A	7	12.7	249	6	80	150	910
SR-B	5	12.7	249	8	80	150	1210

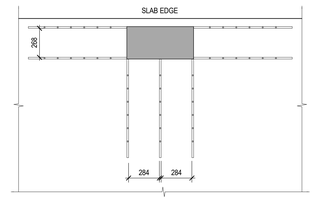
1. STUD RAIL LOCATIONS ARE INDICATED "SR-X" ON PLANS.
2. STUD RAIL SHEAR REINFORCEMENT TO BE MANUFACTURED BY DECON OR APPROVED EQUIVALENT.
3. SHEAR STUDS SHALL CONFORM TO THE FOLLOWING:
   1. LOW CARBON STEEL, C1015 IN ACCORDANCE WITH ASTM-A108
   2. YIELD STRENGTH = 345 MPa MIN.
   3. TENSILE STRENGTH = 414 MPa MIN.
   4. ELONGATION IN 50 mm = 20% MIN.
   5. REDUCTION OF AREA = 50% MIN.
4. RAILS SHALL CONFORM TO THE FOLLOWING: a) LOW CARBON STEEL G40.21M, 300W
5. HEAR STUDS TO BE WELDED IN ACCORDANCE WITH CSA STANDARD W59-1982 AS CERTIFIED BY THE CANADIAN WELDING BUREAU.
6. ALL STUD RAIL SHEAR REINFORCEMENT STRIPS MUST BE SECURELY ATTACHED TO THE FORMWORK USING THE PLASTIC CHAIRS PROVIDED. THE REINFORCEMENT STRIPS ARE NOT TO BE DISPLACED DURING THE PLACEMENT OF THE CONCRETE.
7. EXTERIOR AND INTERIOR CORNER STUD RAILS TO BE LINED UP WITH THE COLUMN FACE WHERE POSSIBLE, PROVIDED THERE IS 50 SPACING BETWEEN ANY SLAB EDGE. PROVIDE 50 SPACING FROM EDGE OF ANY SLAB OPENING OR SLAB THICKNESS CHANGE.



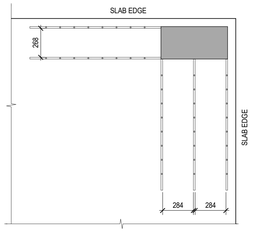
TYPICAL STUD RAIL DETAIL



TYPICAL STUD RAIL POSITION IN SLAB



SR-A



SR-B

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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No	Date	Revision/Issued
1	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

consultants

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structural engineer — LEA Consulting Ltd.
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2026-07-22
E.U.CHOWDHURY
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THE STERLING HALL SCHOOL

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SCHEDULES

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S311
	Revision: 1

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

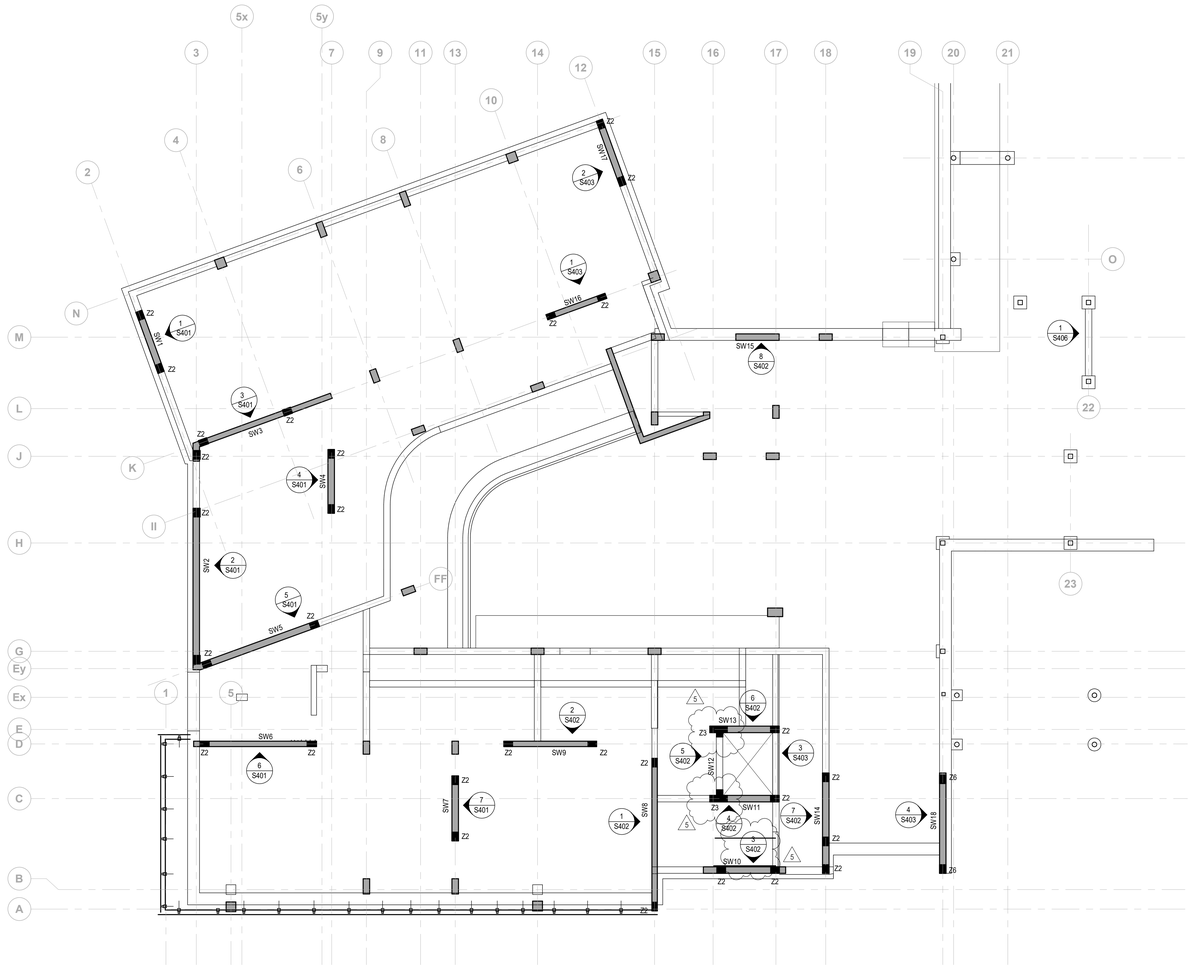
DRAWING LAST EDITED: 2026-07-22 3:08:50 PM

CONCRETE SHEAR WALL SCHEDULE

LEVEL / SW#	SW1	SW2	SW3	SW4	SW5	SW6	SW7	SW8	SW9
HIGH ROOF 11350									
ROOF 11050									
ELEV. 10400									
SECOND FLOOR 6600	300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF
MEZZANINE 3000									
UPPER GROUND FLOOR 1800									
GROUND FLOOR 0	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION				
NATIVE SOIL -2000									
TOP OF FOUNDATION LEVEL -3000									
REMARKS									

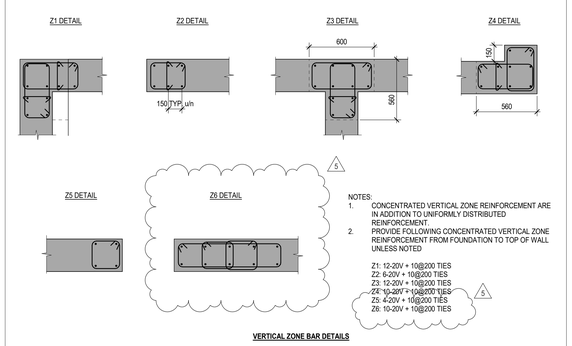
CONCRETE SHEAR WALL SCHEDULE

LEVEL / SW#	SW10	SW11	SW12	SW13	SW14	SW15	SW16	SW17	SW18
HIGH ROOF 11350									
ROOF 11050									
ELEV. 10400									
SECOND FLOOR 6600	ELEV. 5840	300 WALL 15@300 VEF 15@300 HEF	250 WALL 15@325 VEF 15@325 HEF	250 WALL 15@325 VEF 15@325 HEF	ELEV. 6018		250 WALL 15@325 VEF 15@325 HEF		ELEV. 6018
MEZZANINE 3000	300 WALL 15@300 VEF 15@300 HEF				300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@400 VEF 15@400 HEF		300 WALL 15@300 VEF 15@300 HEF	300 WALL 15@300 VEF 15@300 HEF
UPPER GROUND FLOOR 1800									
GROUND FLOOR 0	FOUNDATION WALL; SEE PLAN AND SECTION				FOUNDATION WALL; SEE PLAN AND SECTION	FOUNDATION WALL; SEE PLAN AND SECTION		FOUNDATION WALL; SEE PLAN AND SECTION	
NATIVE SOIL -2000									
TOP OF FOUNDATION LEVEL -3000									
REMARKS									

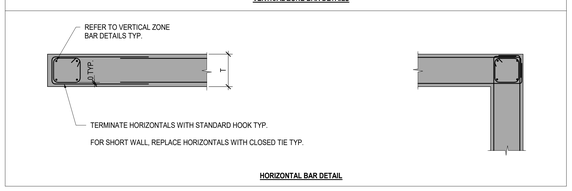


1 S400 GROUND FLOOR SHEAR WALL KEY PLAN
1 : 100

CONCRETE SHEAR WALL TYPICAL DETAILS



VERTICAL ZONE BAR DETAILS



HORIZONTAL BAR DETAIL

CONCRETE SHEAR WALL SCHEDULE NOTES

1. FOR GENERAL REQUIREMENTS AND TYPICAL DETAILS SEE DRAWINGS S100 SERIES.
2. FOR LOCATIONS OF SCHEDULED SHEAR WALLS, SEE KEY PLAN.
3. CONCENTRATED VERTICAL REINFORCEMENT IS IN ADDITION TO TYPICAL DISTRIBUTED REINFORCEMENT. CONCENTRATED VERTICAL REINFORCED TO BE SPACED AT 150, UNLESS NOTED OTHERWISE, BUT NOT LESS THAN 100 APART, CENTRE TO CENTRE. CONCENTRATED VERTICAL REINFORCEMENT REFERS TO VERTICAL WALL REINFORCEMENT TIED AS A COLUMN.
4. SEE FOUNDATION PLANS AND SECTIONS FOR ADDITIONAL REINFORCEMENT AT AND BELOW THE GRADE LEVEL.
5. CARRY ALL WALLS AND REINFORCEMENT TO TOP OF FOOTINGS.
6. PROVIDE "L" DOWELS FROM WALL INTO FLOOR SLABS TO MATCH SIZE AND SPACING OF TYPICAL DISTRIBUTED WALL REINFORCEMENT UNLESS NOTED OTHERWISE. PROVIDE CLASS 'B' TENSION LAP WITH SLAB AND WALL STEEL REINFORCEMENT.
7. PROVIDE DOWELS FROM FOOTING TO WALL TO MATCH THE CONCENTRATED AND DISTRIBUTED VERTICAL WALL REINFORCEMENT. ALL DOWELS TO EXTEND TO THE BOTTOM OF THE MAT FOOTING. PROVIDE CLASS 'B' TENSION LAP WITH WALL REINFORCEMENT ABOVE. DOWELS FOR THE CONCENTRATED VERTICAL REINFORCEMENT SHALL TERMINATE WITH A 90 DEGREE HOOK.
8. IF TIES ARE SHOWN FOR CONCENTRATED VERTICAL REINFORCEMENT BARS, EVERY CORNER AND ALTERNATE LONGITUDINAL BAR SHALL HAVE LATERAL SUPPORT PROVIDED BY THE CORNER OF A TIE HAVING AN INCLUDED ANGLE OF NOT MORE THAN 135 DEGREE. BEND TYPICAL HORIZONTAL BARS, CARRY INTO CONCENTRATED VERTICAL REINFORCEMENT BAR CAGE AND HOOK AT FAR FACE.
9. PROVIDE CLASS 'B' TENSION LAPS FOR HORIZONTAL AND VERTICAL REINFORCEMENT.

Key to Detail Location

NO. / NO. — Detail Number / Drawing Number

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5	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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THE STERLING HALL SCHOOL

99 Cartwright Avenue, Toronto, ON M6A 1V4

SHEAR WALL KEY PLAN AND SCHEDULE

scale:	As indicated
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S400 Revision 5

A0 FILENAME: Autodesk Docs://2022 Sterling Hall\22317_SHS-STR-R25.rvt

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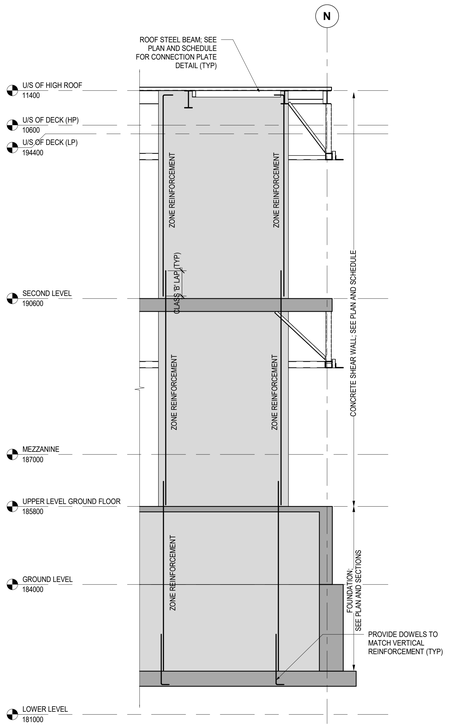

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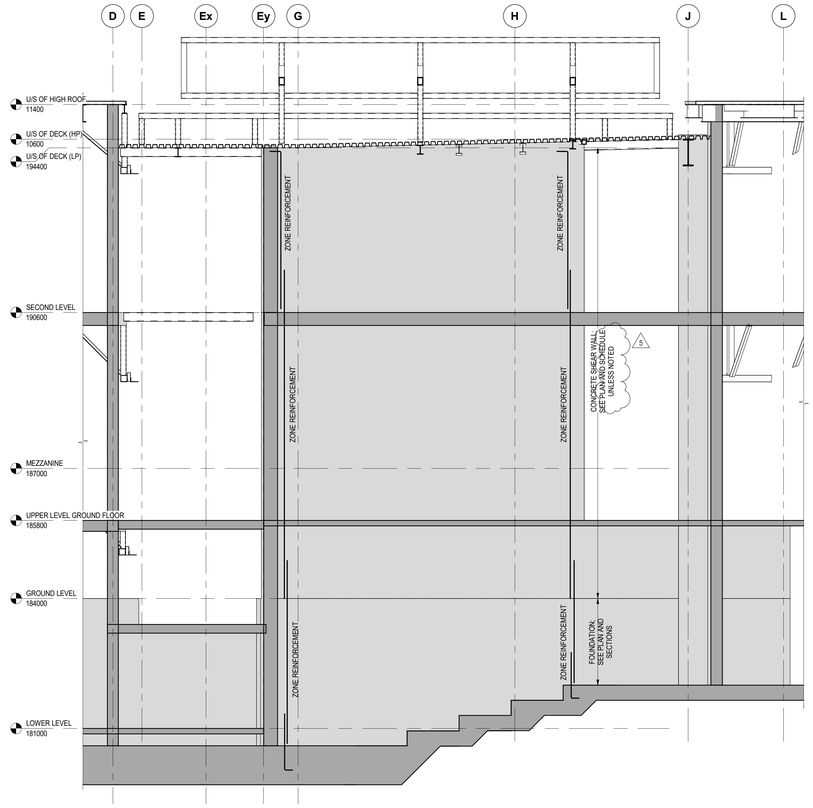

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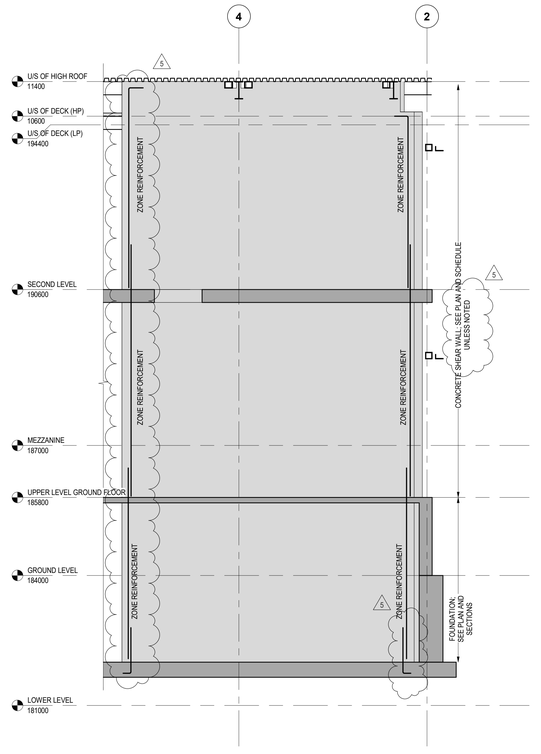

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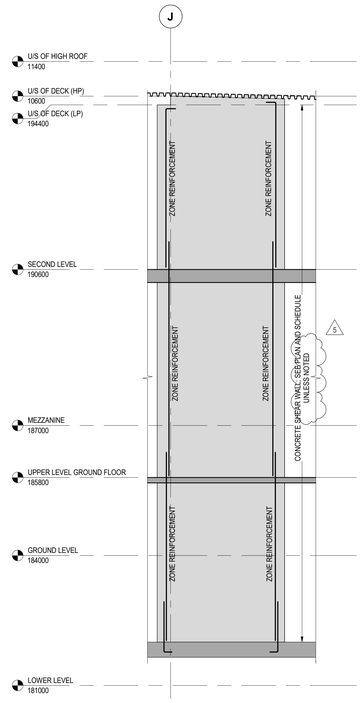

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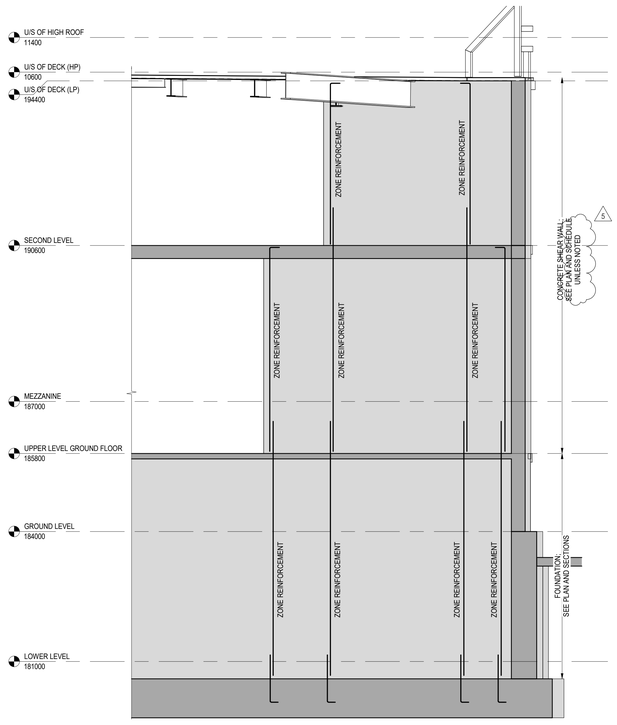

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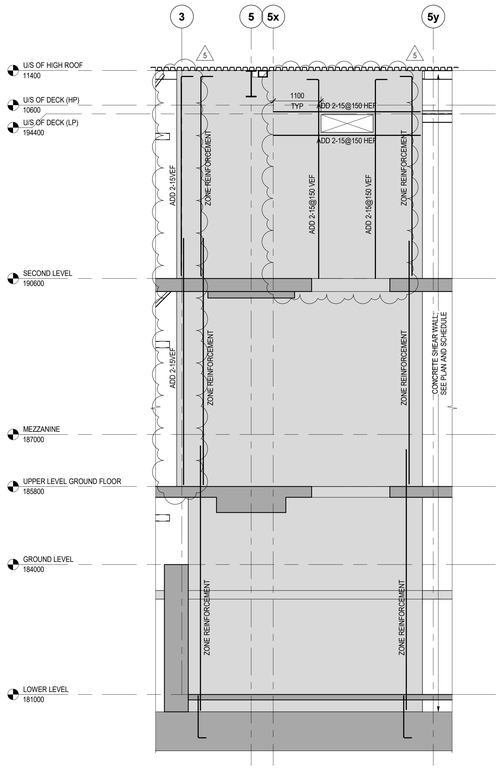

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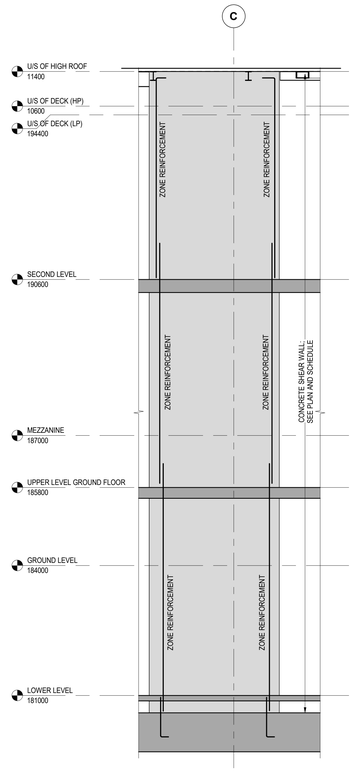

7 S401 SW7 ELEVATION
1 : 50

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NO. Drawing Number

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S401
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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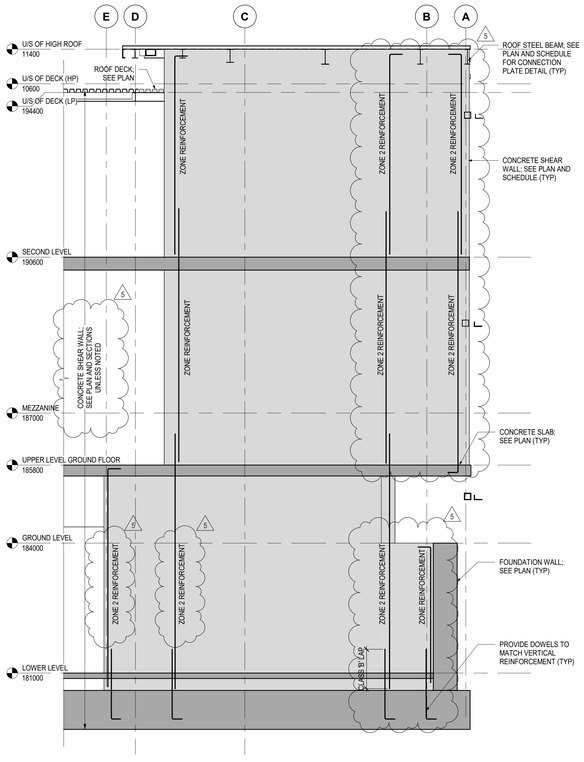

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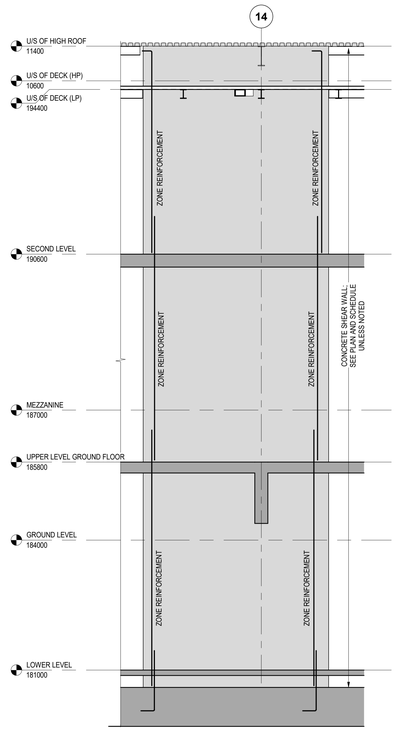

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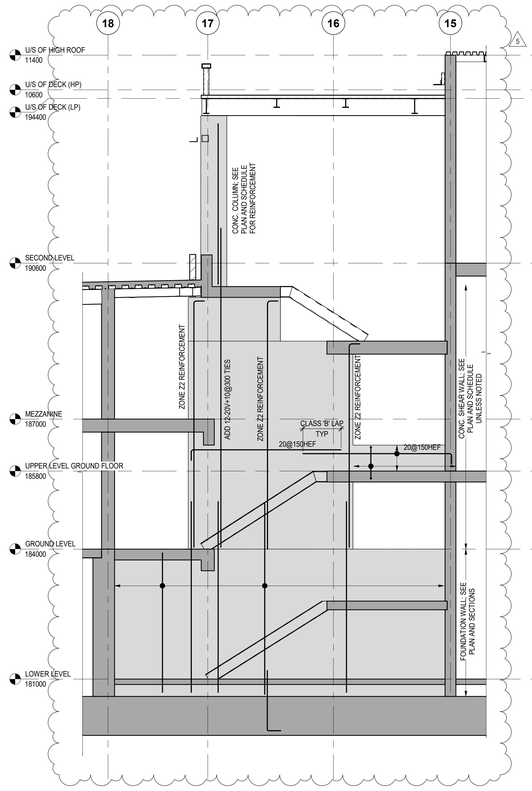

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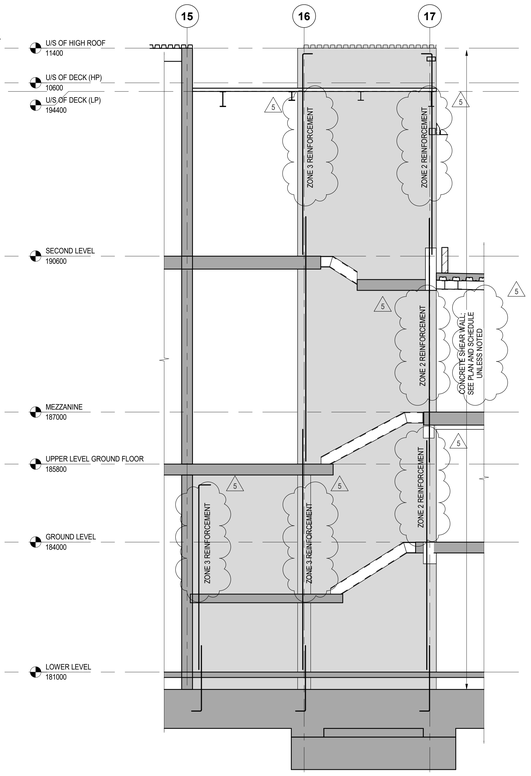

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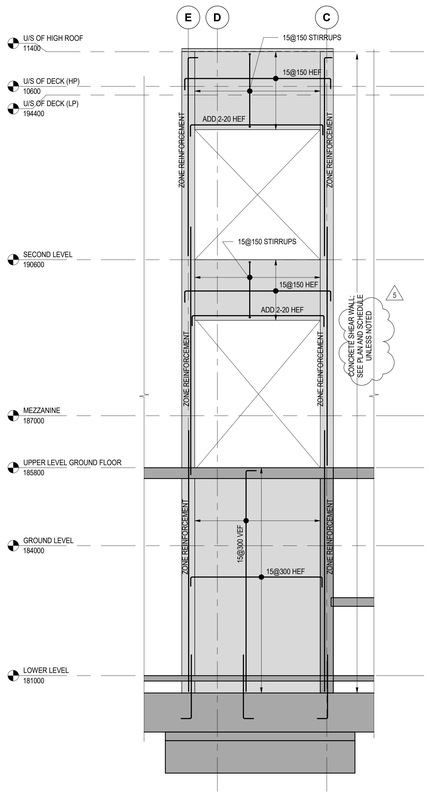

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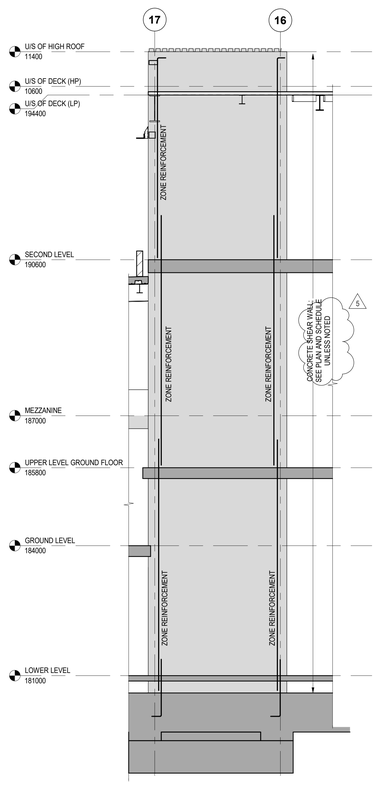

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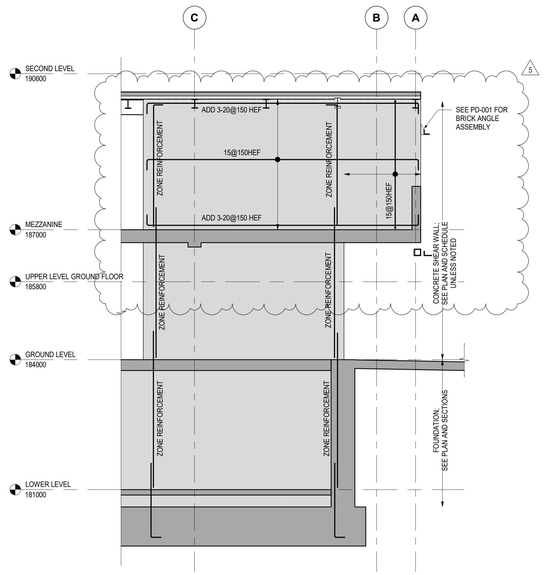

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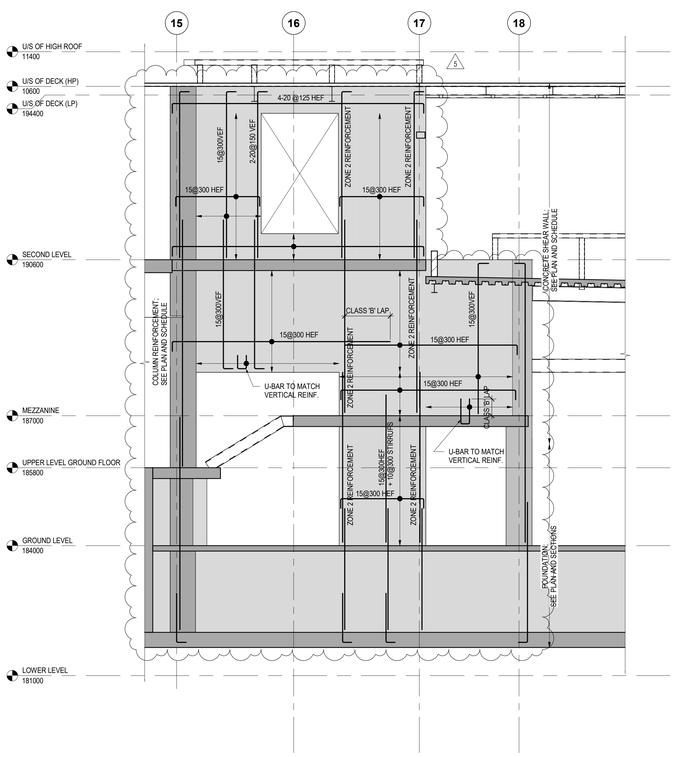

8 S402 ELEVATION
1 : 50

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THE STERLING HALL SCHOOL

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S402
Revision:	5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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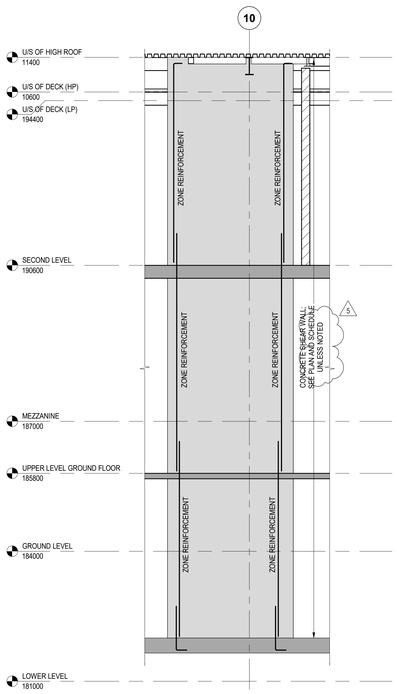

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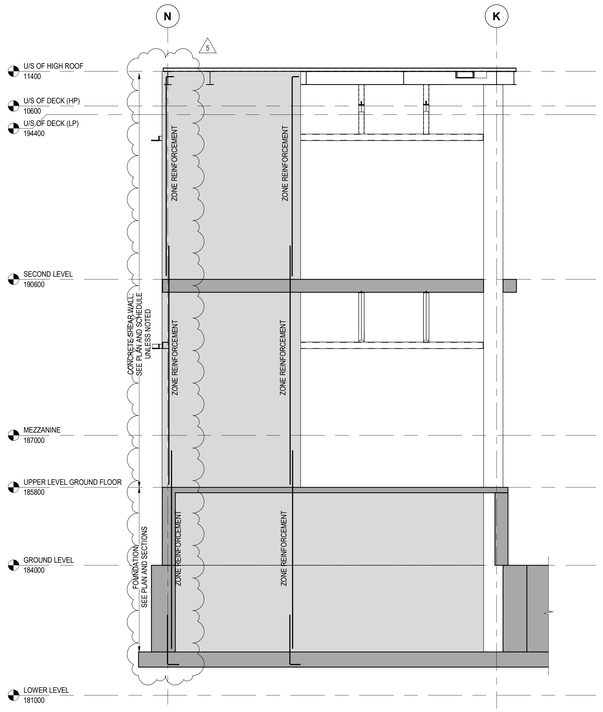

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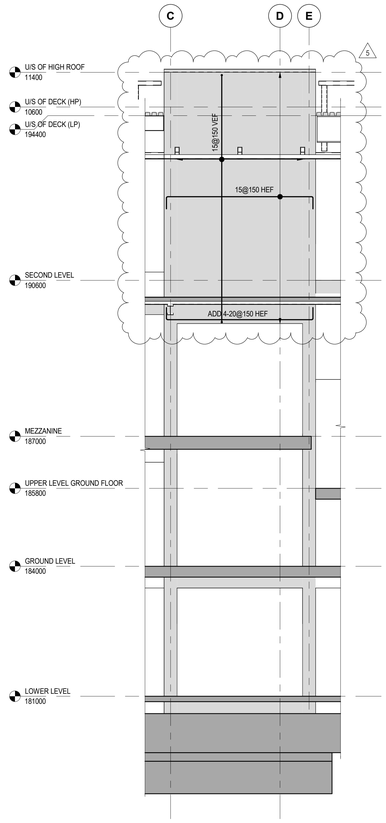

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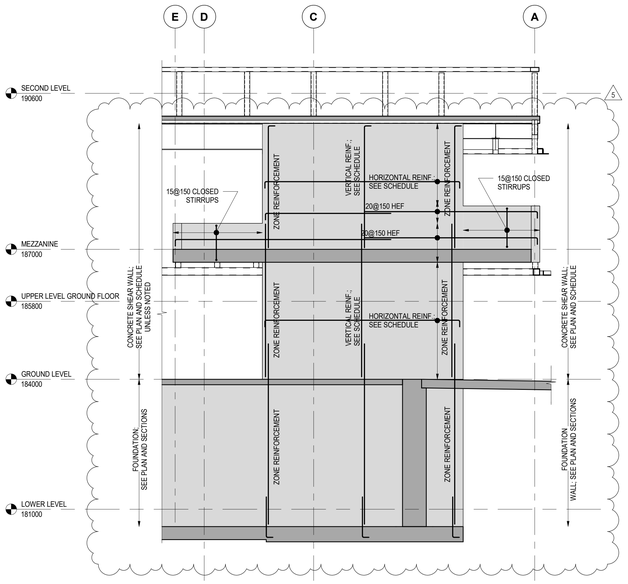

4 S403 ELEVATION
1 : 50

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THE STERLING HALL SCHOOL

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SHEAR WALL ELEVATIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S403
	Revision: 5

A0 FILENAME: Autodesk Docs://2103 Sterling Hall/22317_SHS-STR-R25.rvt

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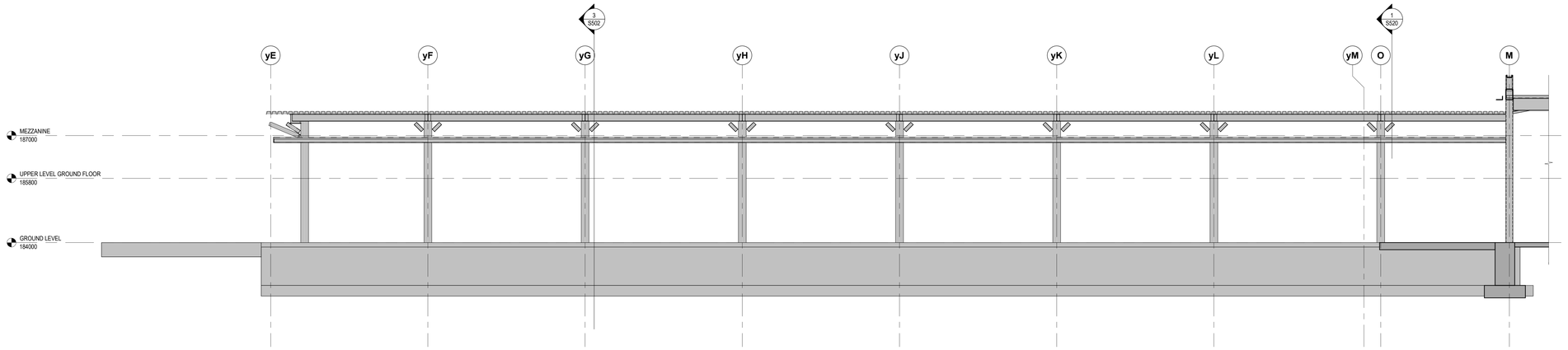

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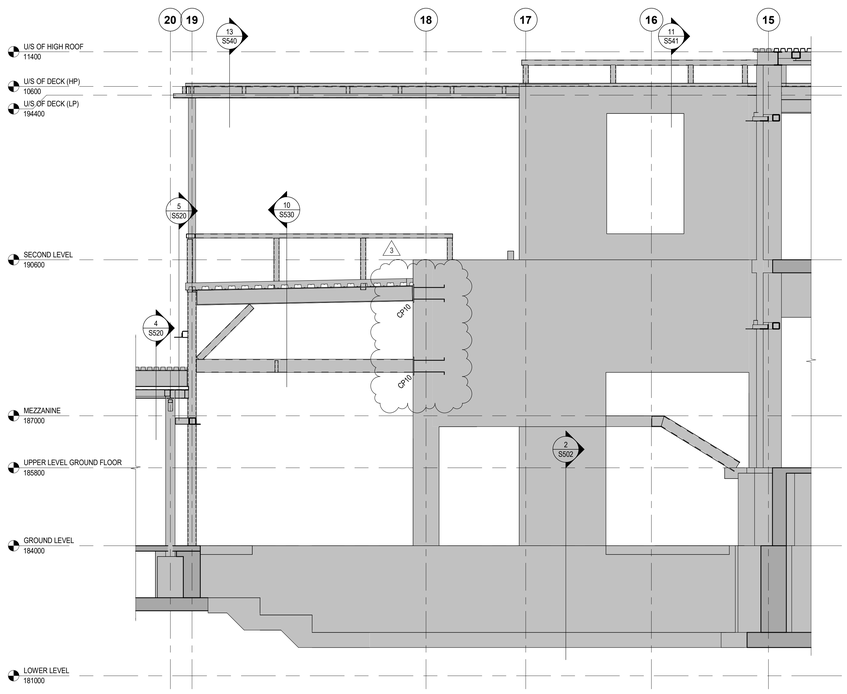

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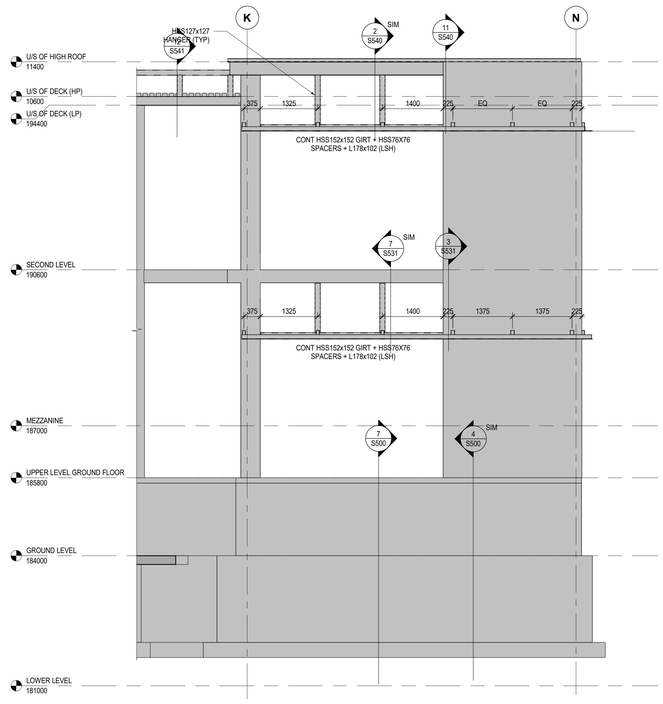

3 S410 ELEVATION
1 : 50

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BUILDING ELEVATIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S410 Revision 3

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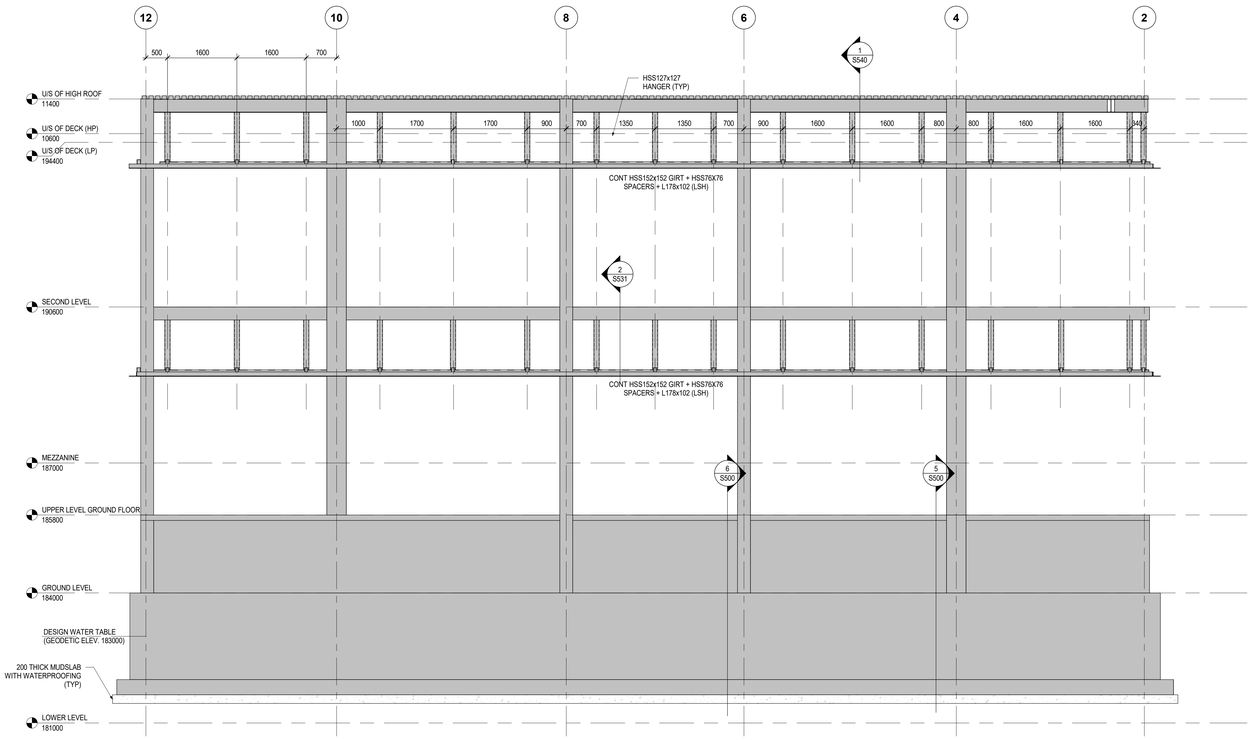

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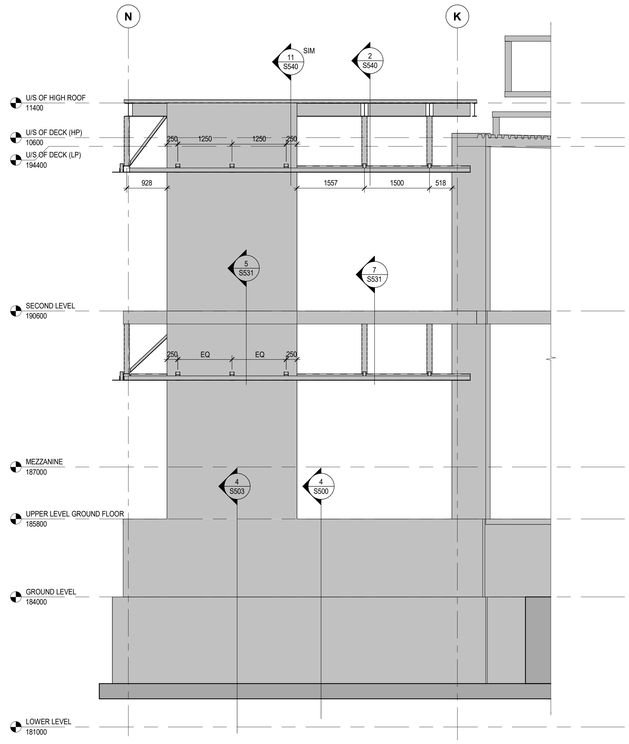

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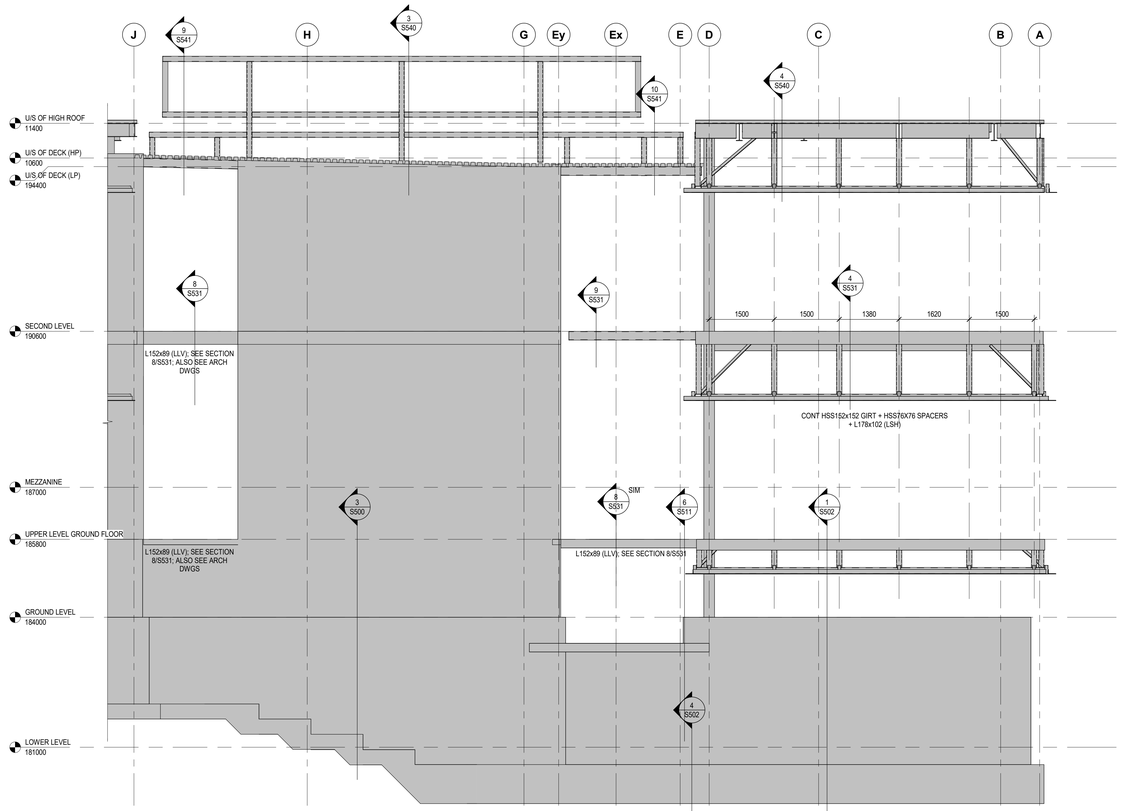

3 S411 ELEVATION
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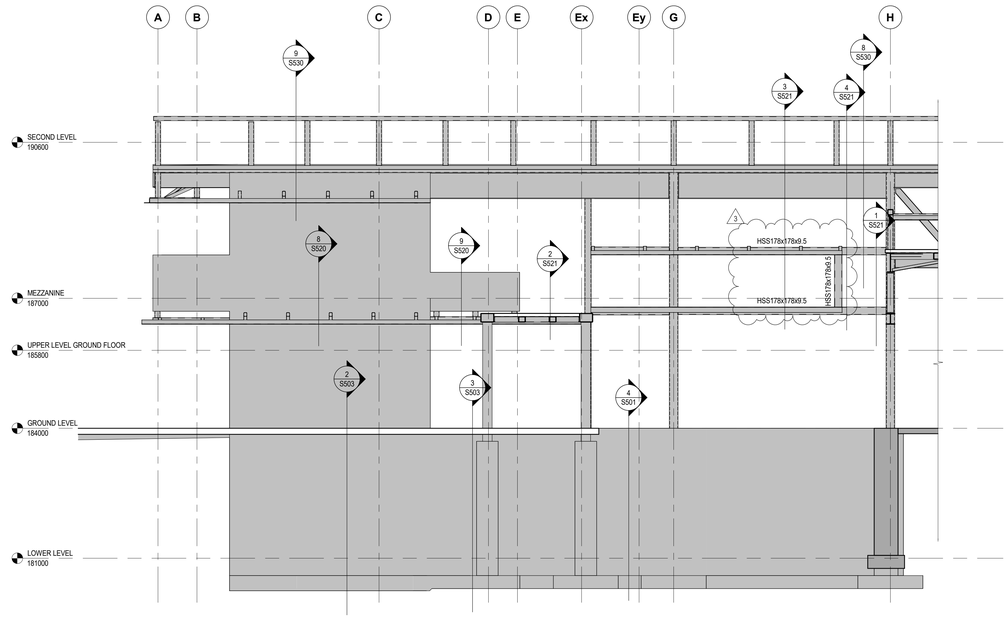

4 S411 ELEVATION
1 : 50

Key to Detail Location

NO. Detail Number
NO. Drawing Number

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2	2026-07-08	ISSUED FOR TENDER
3	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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BUILDING ELEVATIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S411
	Revision: 3

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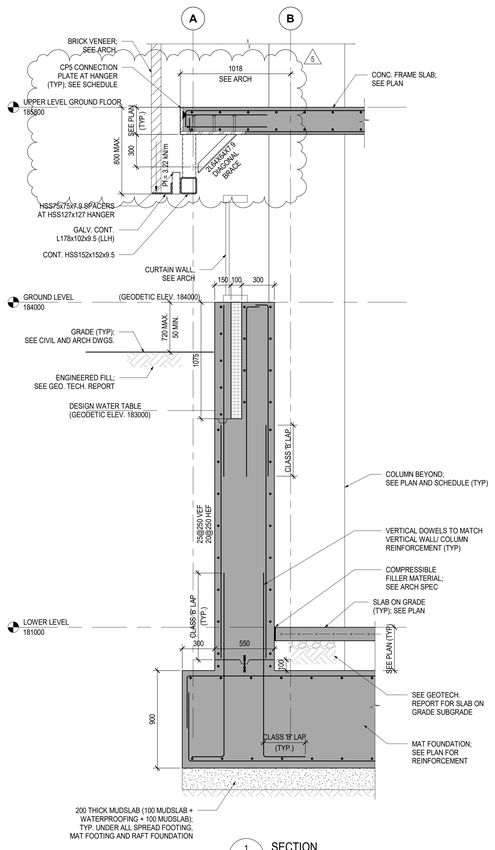

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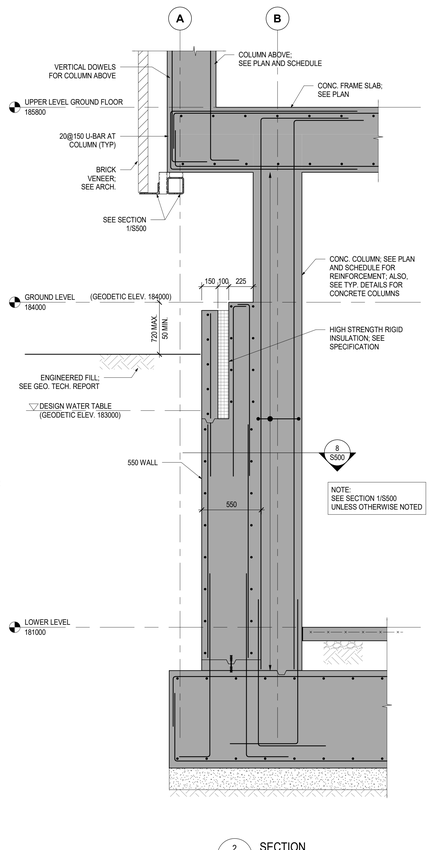

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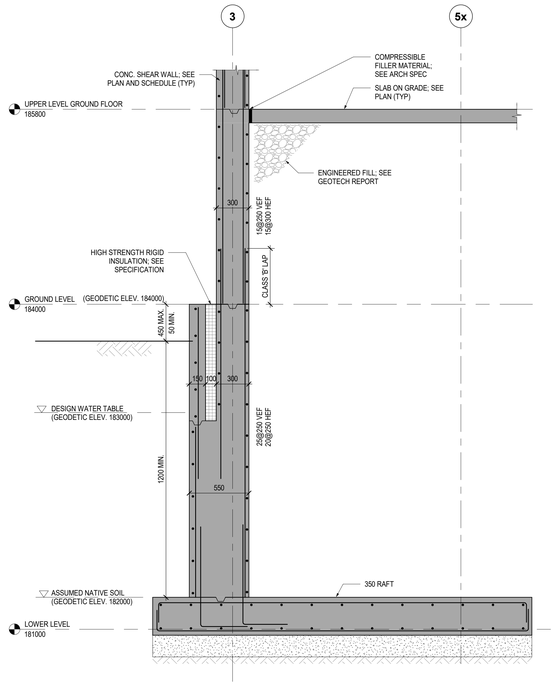

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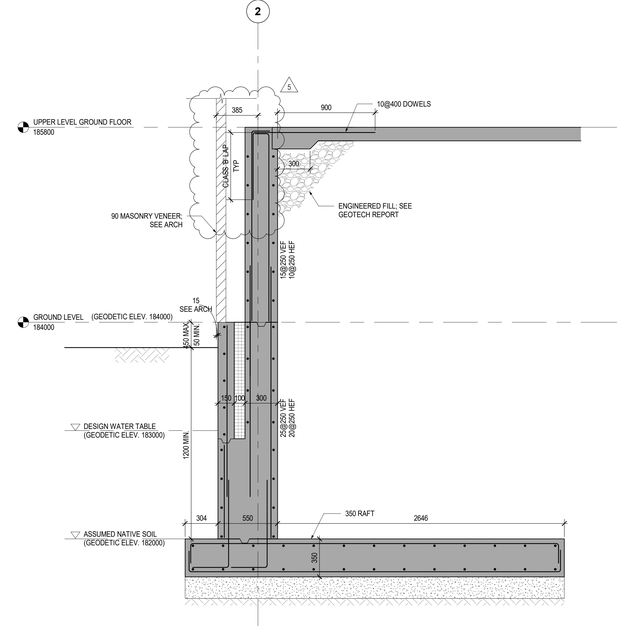

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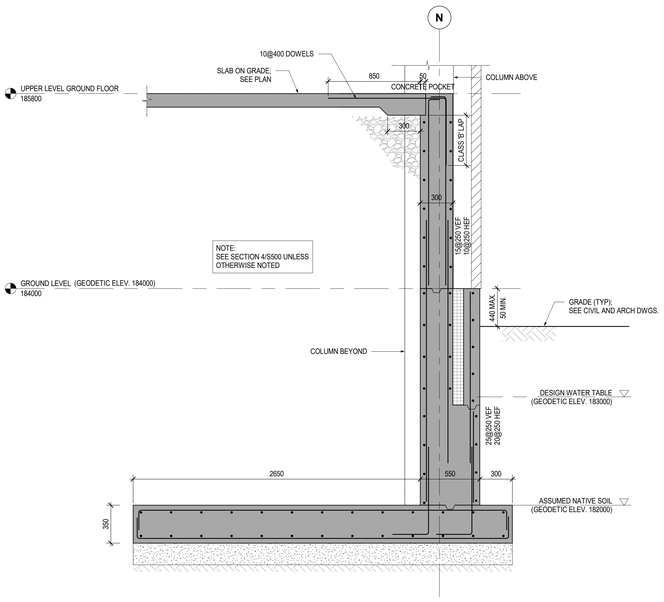

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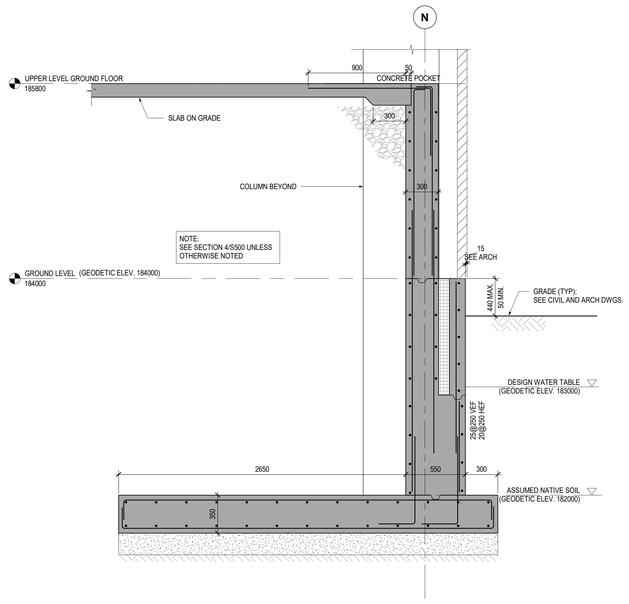

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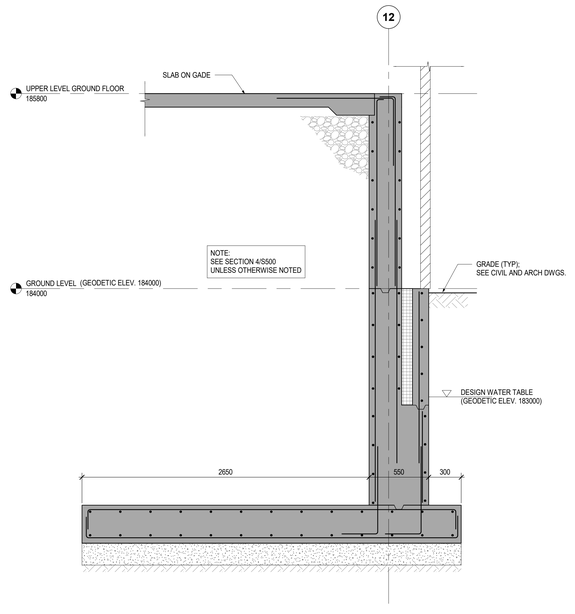

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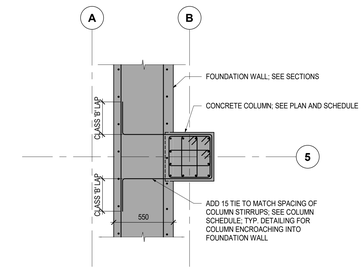

8 S500 PLAN DETAIL 1 : 20

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FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S500
	Revision: 5

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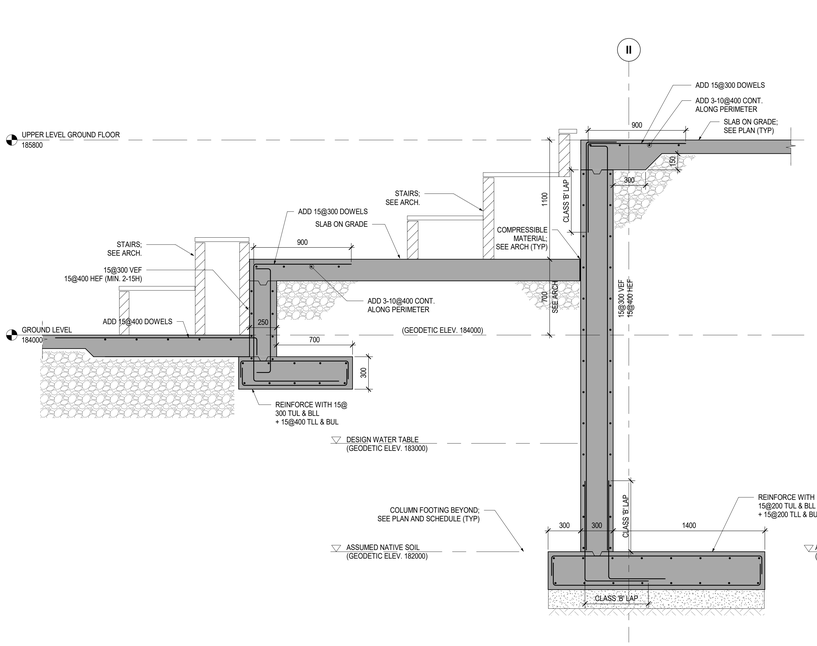

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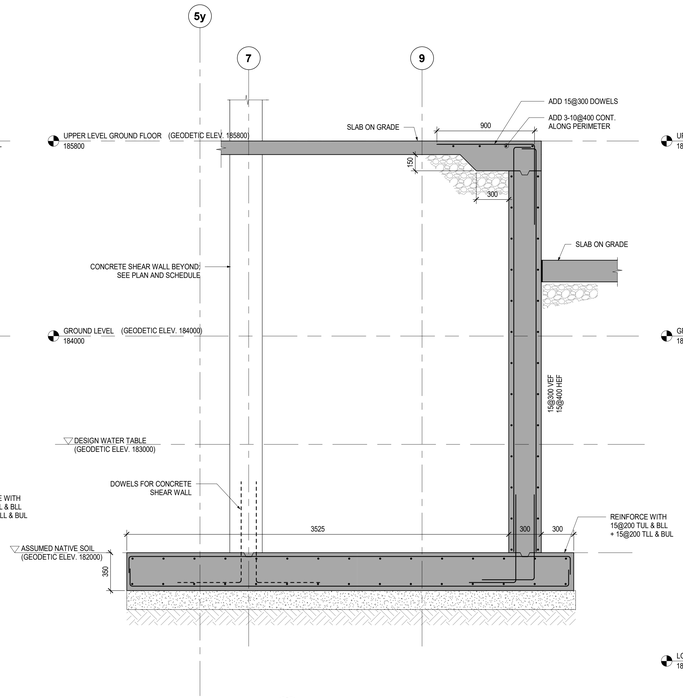

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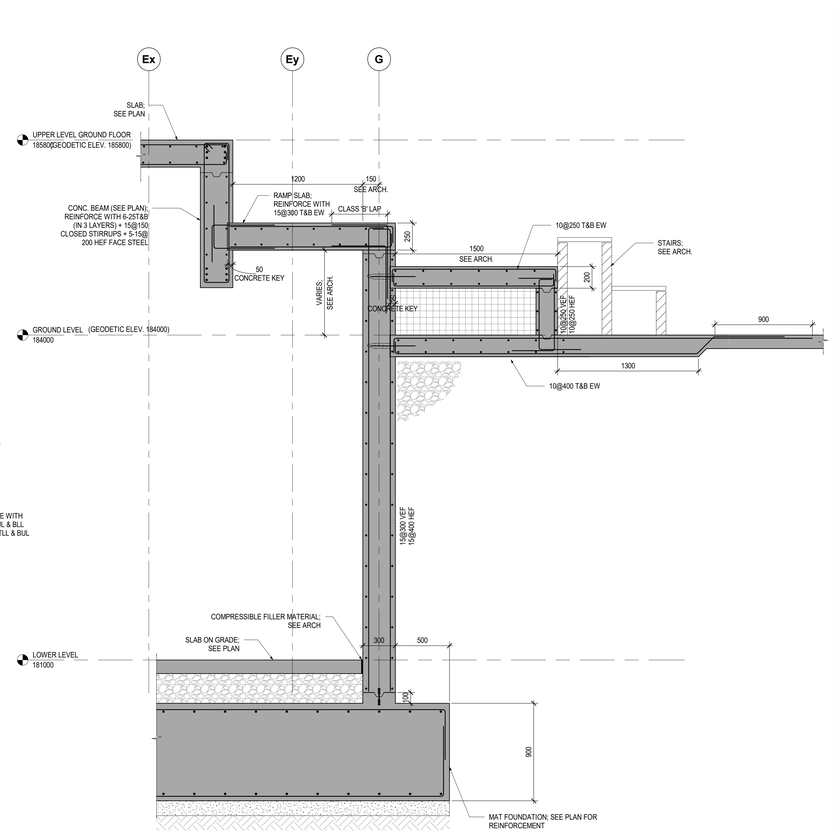

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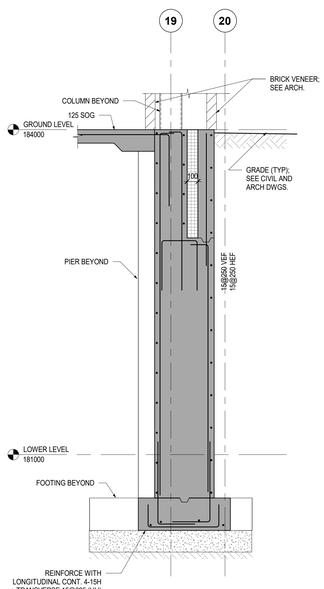

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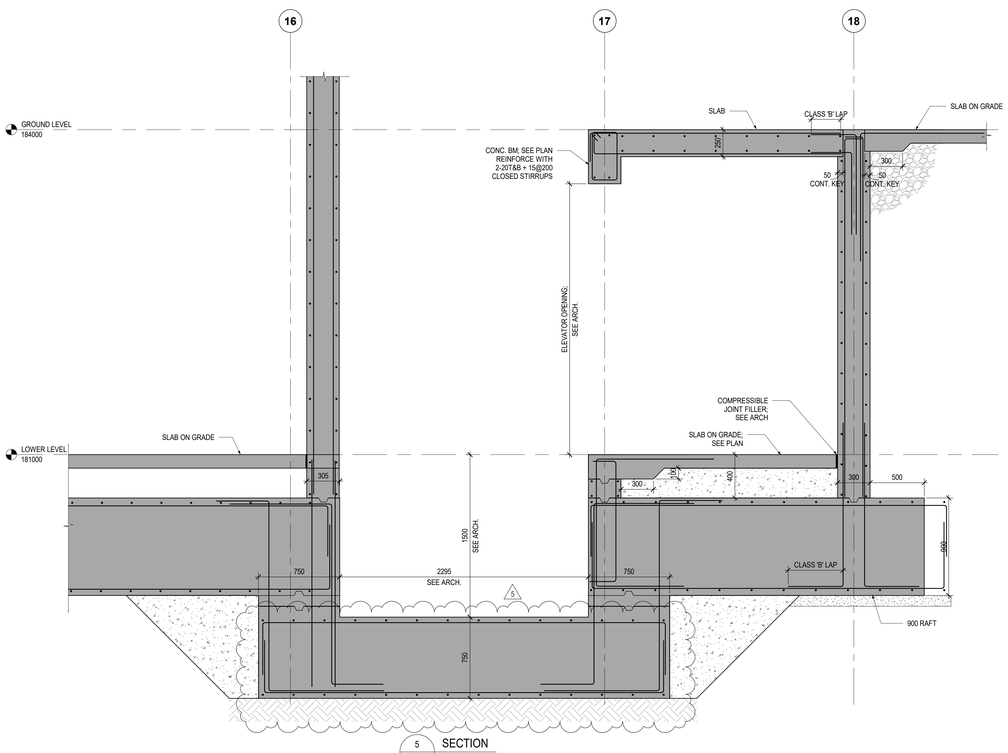

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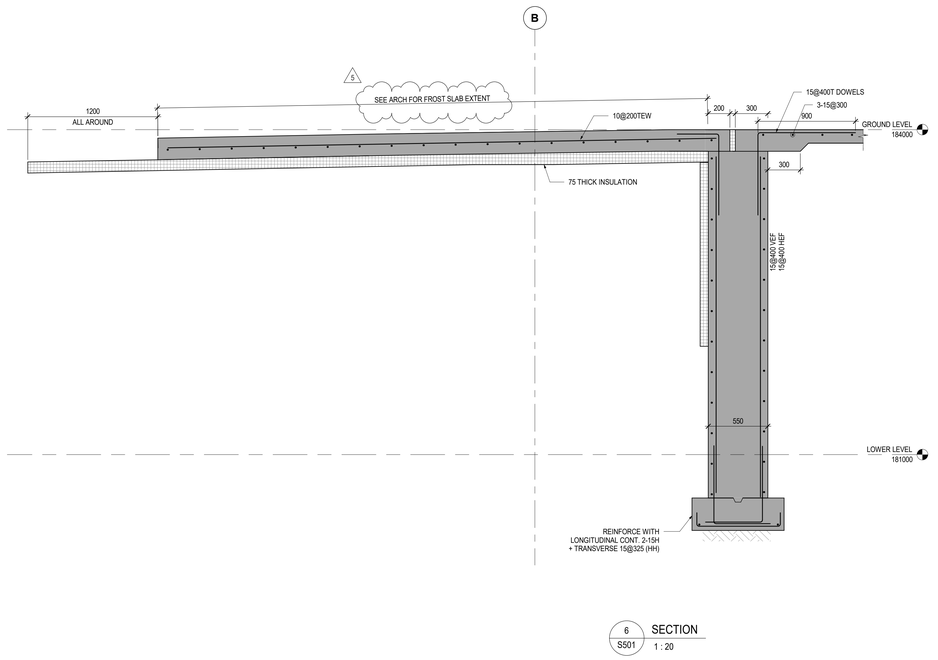

6 S501 SECTION 1 : 20

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FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S501
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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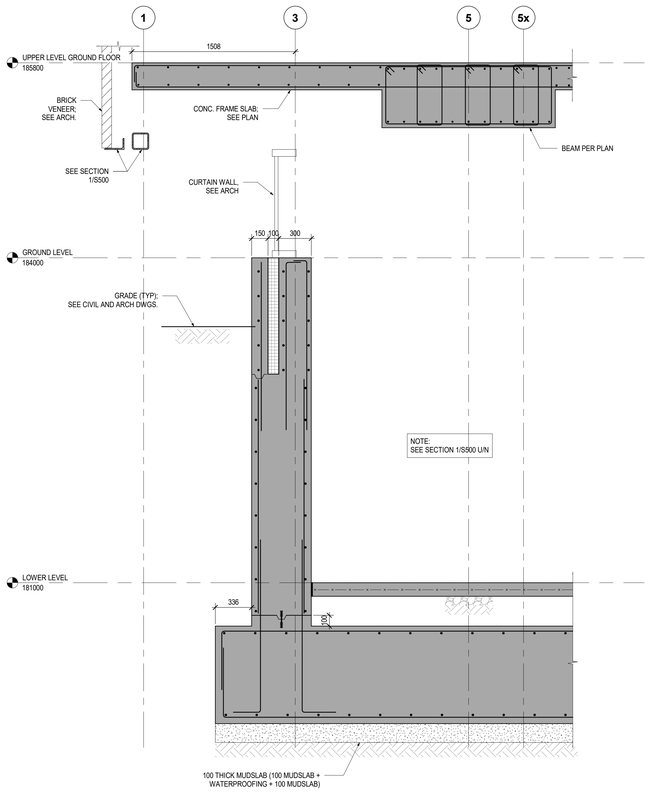

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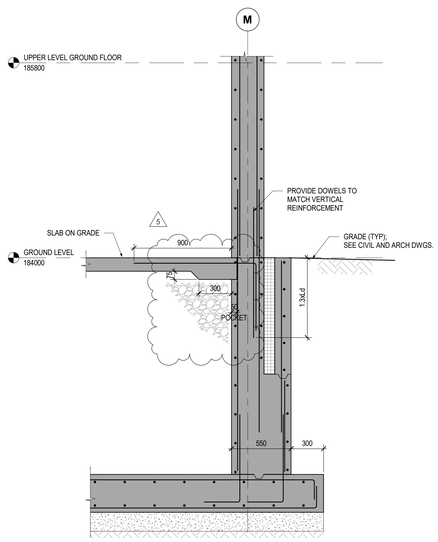

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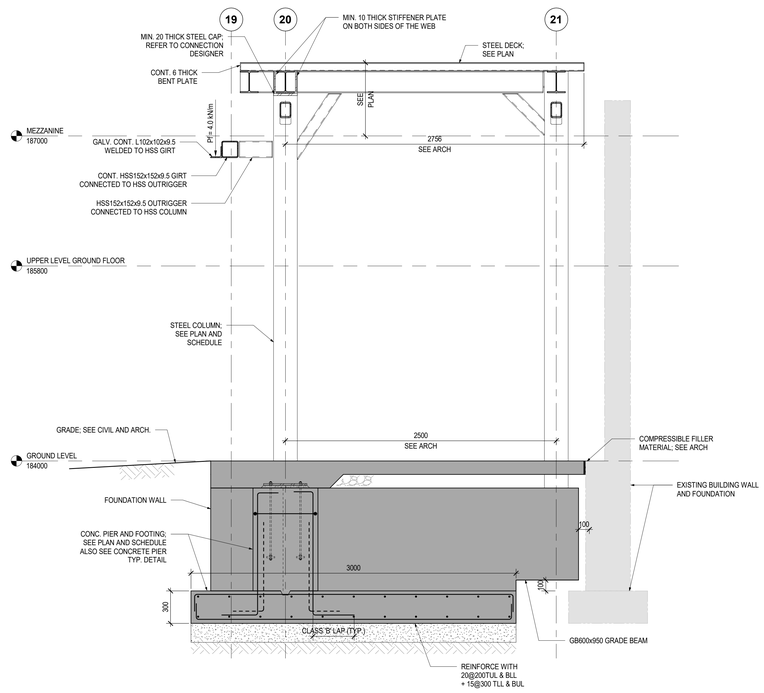
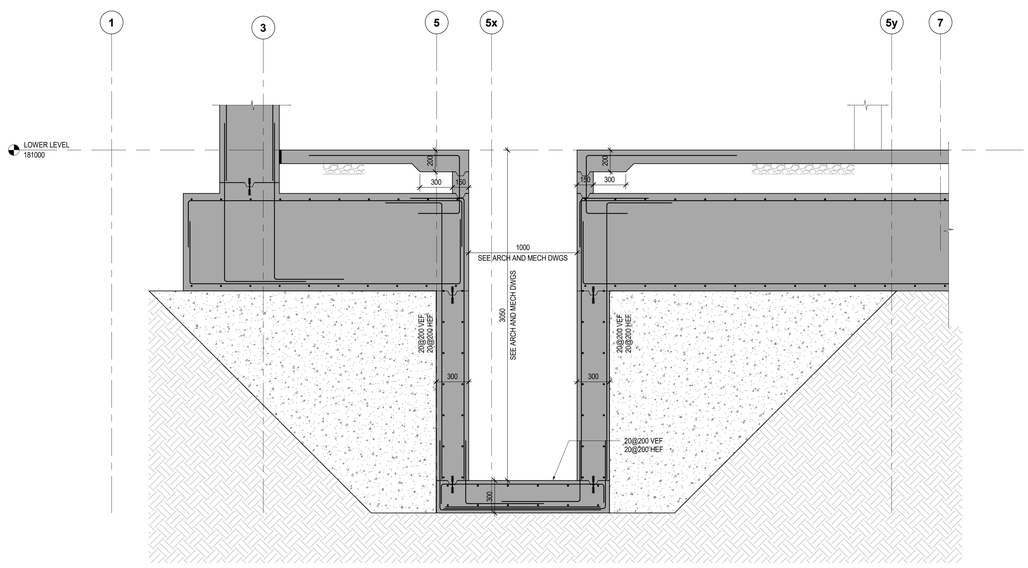


3 / S502 SECTION 1 : 20



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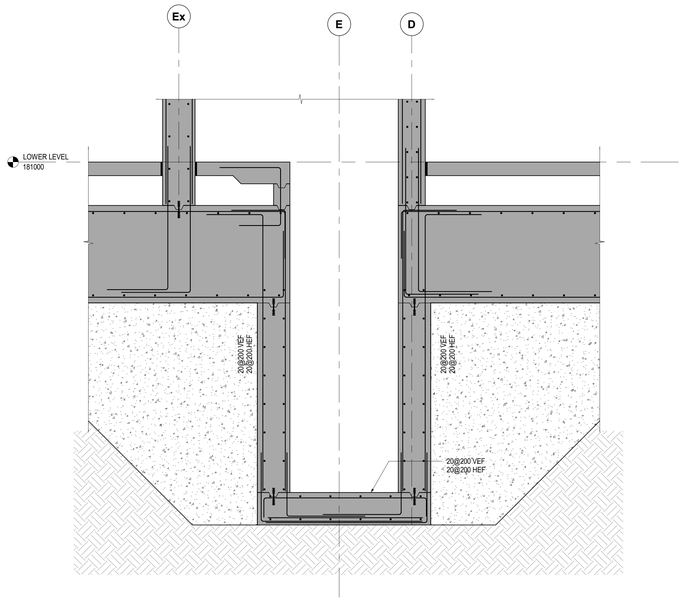

5 / S502 SECTION 1 : 20

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FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S502
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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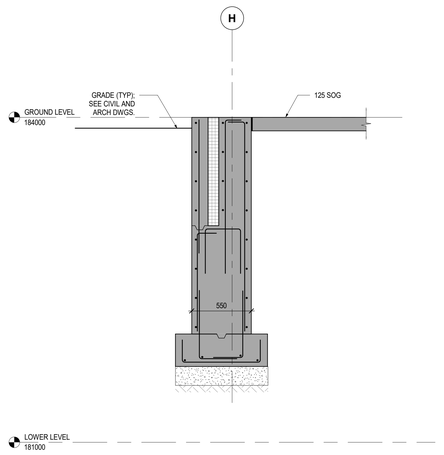
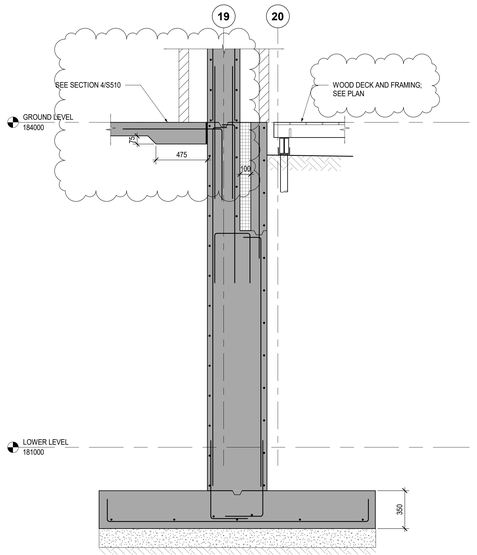


1 / S503 SECTION 1 : 20



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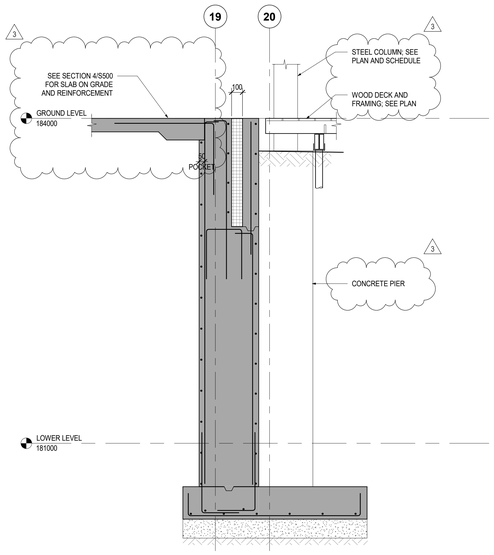

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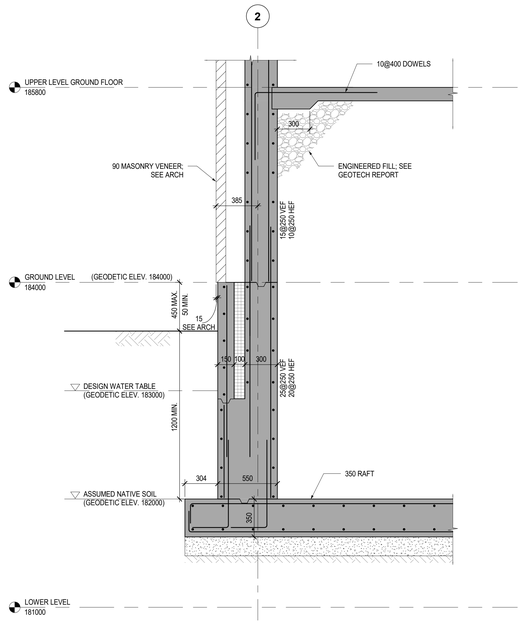

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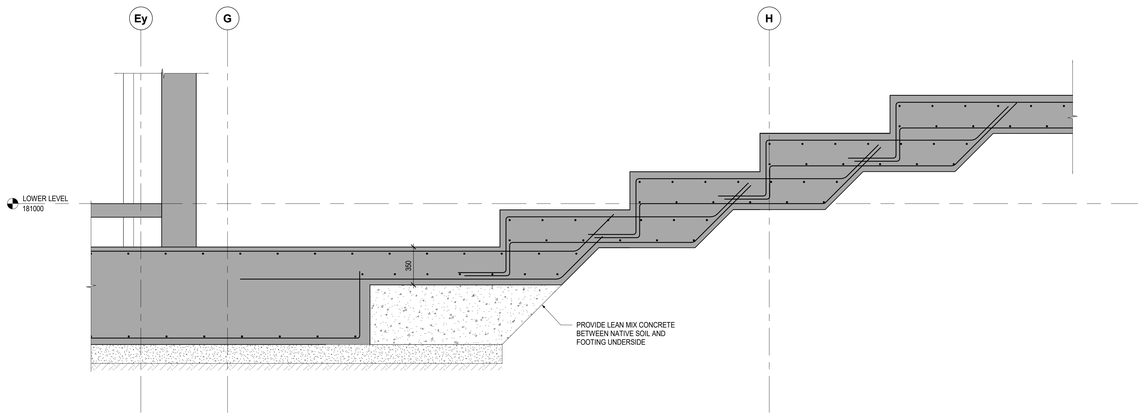

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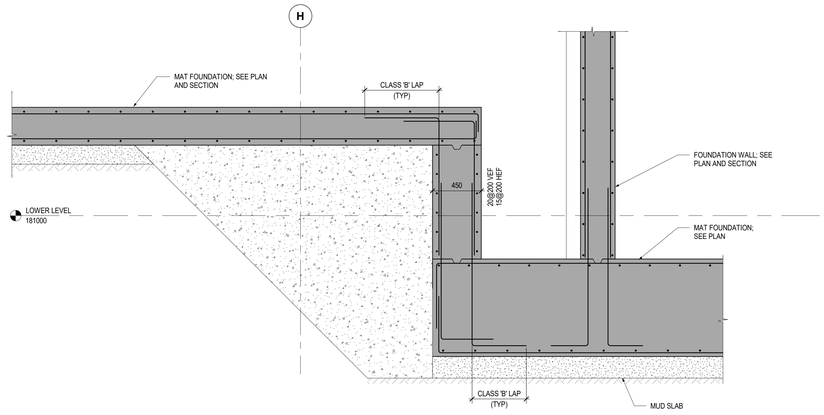

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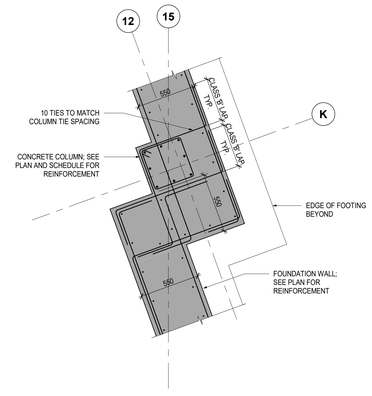

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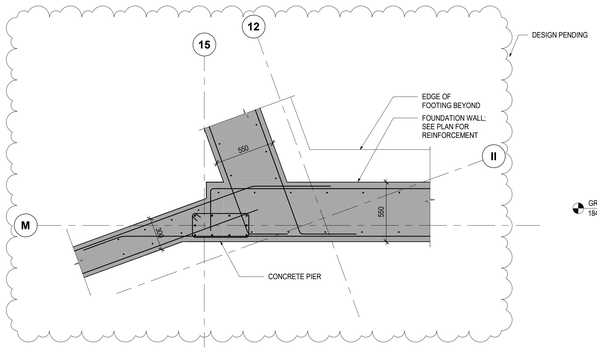

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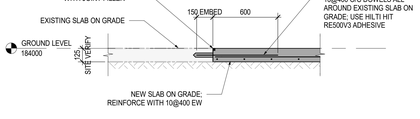
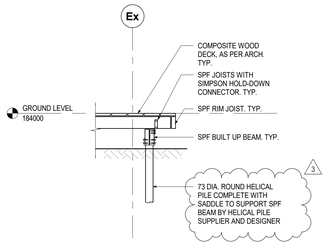


9 / S503 SECTION 1 : 20



10 / S503 SECTION 1 : 20

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FOUNDATION SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S503
	Revision: 3

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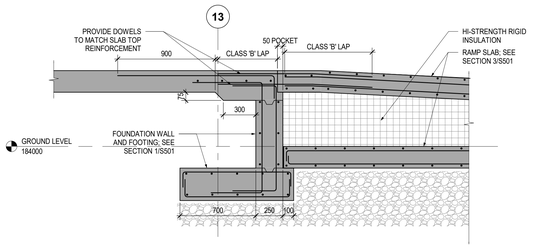
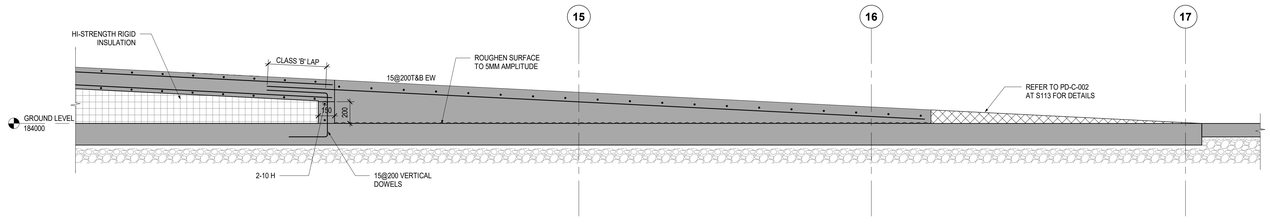


1 SECTION
S510 1 : 20



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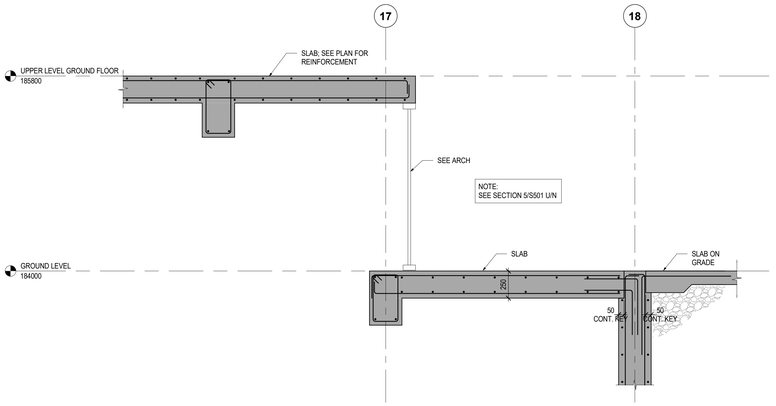

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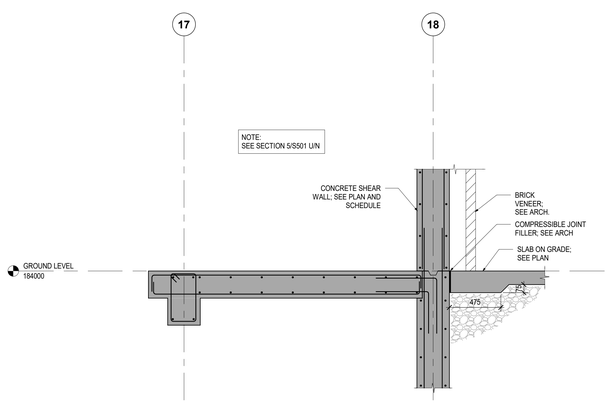
S510 1 : 20



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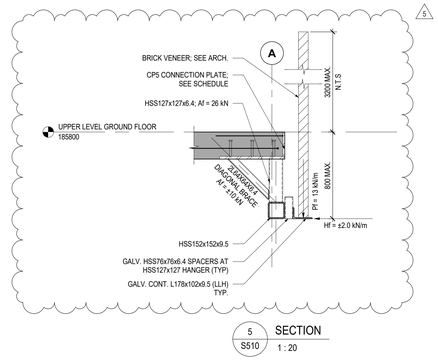
S510 1 : 20



5 SECTION
S510 1 : 20

Key to Detail Location

NO. — Detail Number
NO. — Drawing Number

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4	2026-07-08	ISSUED FOR TENDER
5	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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GROUND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S510
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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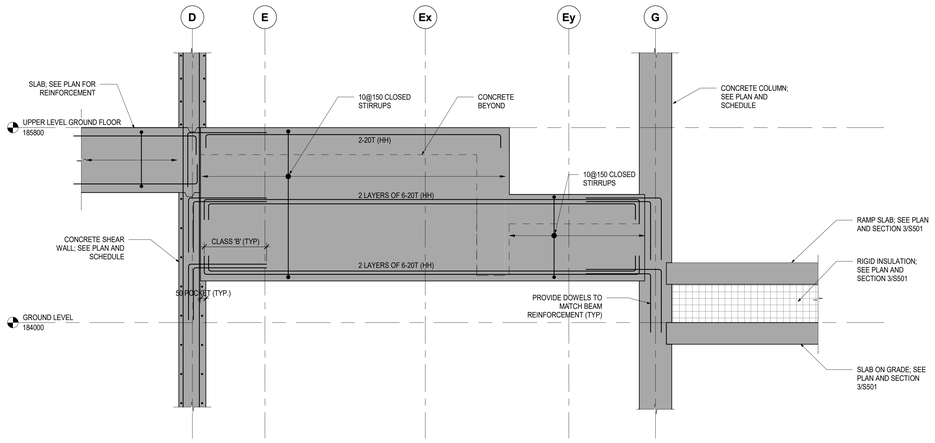

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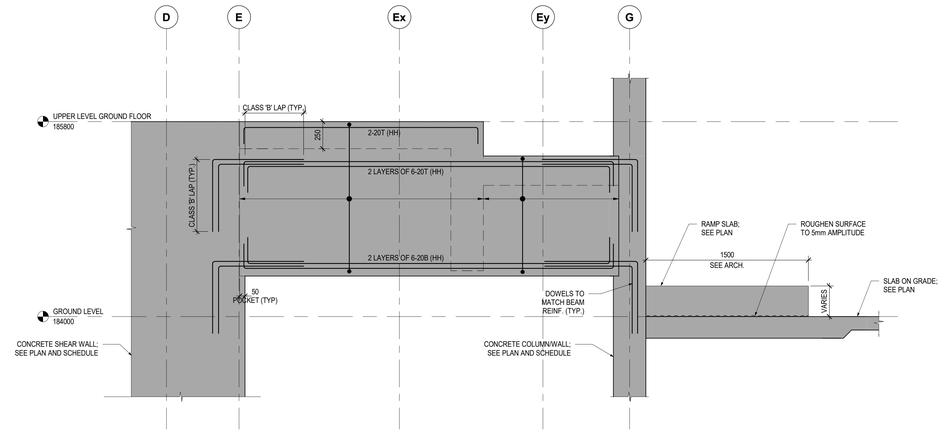

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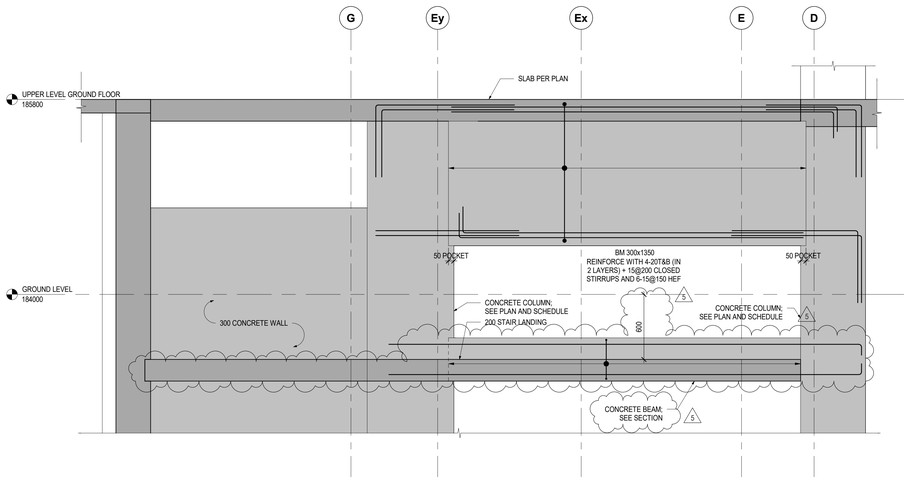
1 : 20



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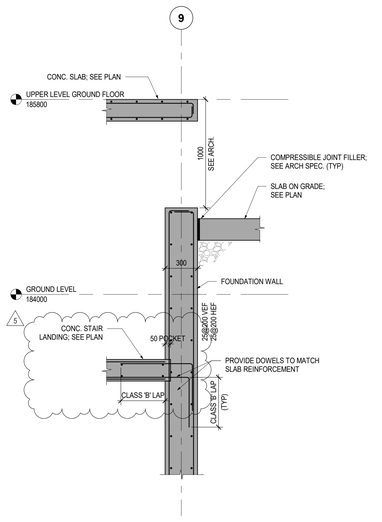

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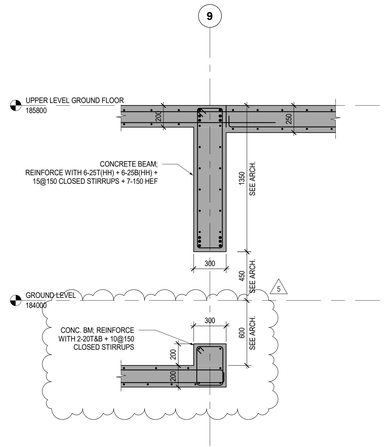

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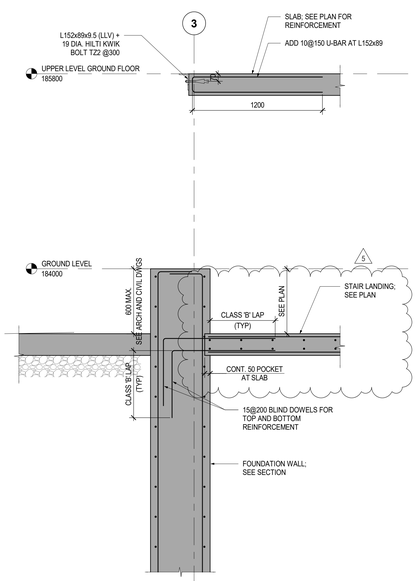

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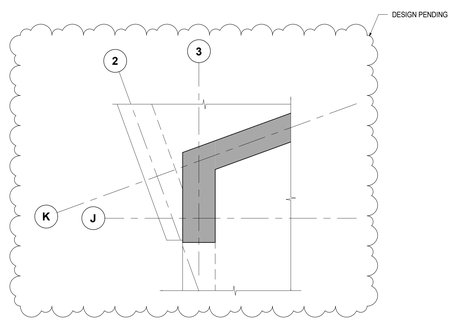

7 / S511 PLAN DETAIL
1 : 20

Key to Detail Location

NO. — Detail Number
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5	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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GROUND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S511
	Revision: 5

A0 FILENAME: Autodesk Docs://2317 Sterling Hall/22317_SHS-STR-R25.rvt

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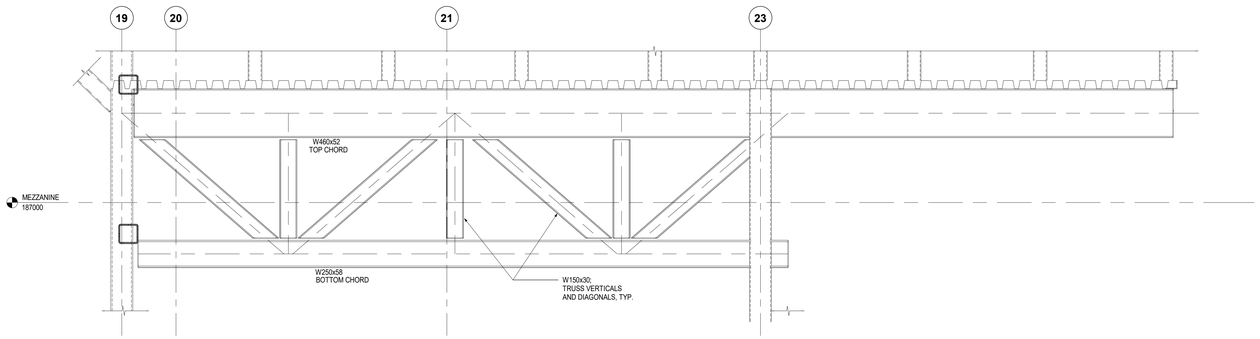

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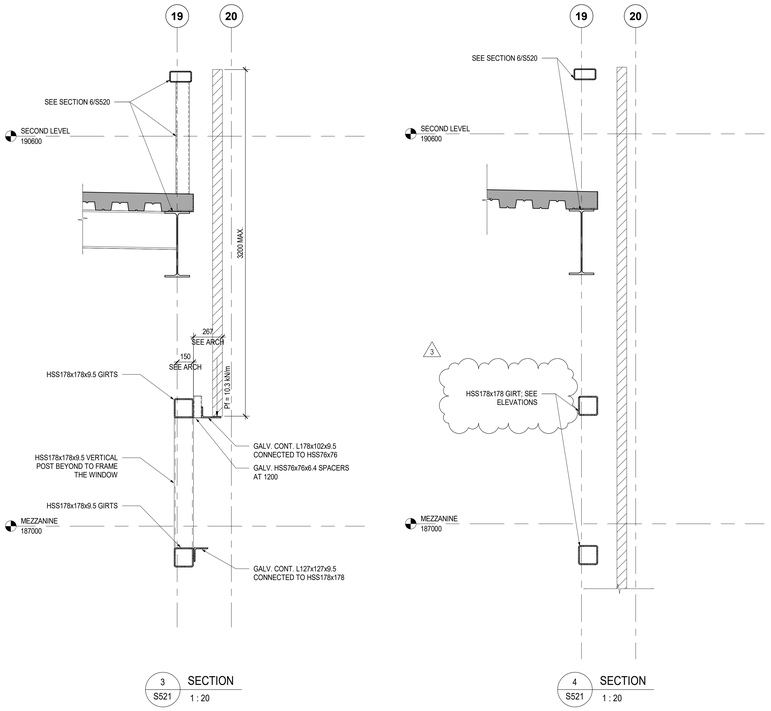
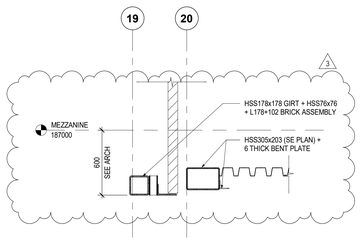


3 / S521 SECTION
1 : 20

4 / S521 SECTION
1 : 20



2 / S521 SECTION
1 : 20

Key to Detail Location

NO. / NO. Detail Number / Drawing Number

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MEZZANINE FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S521
Revision:	3

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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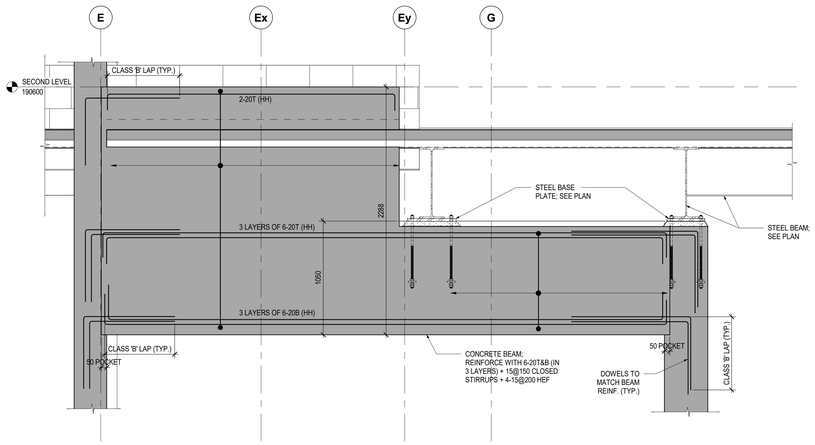

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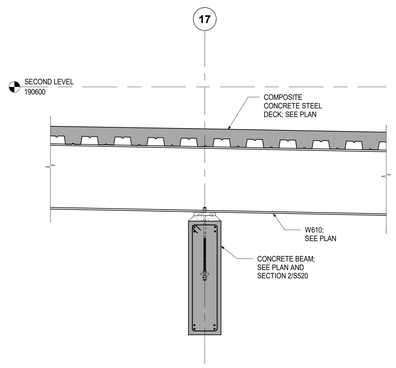

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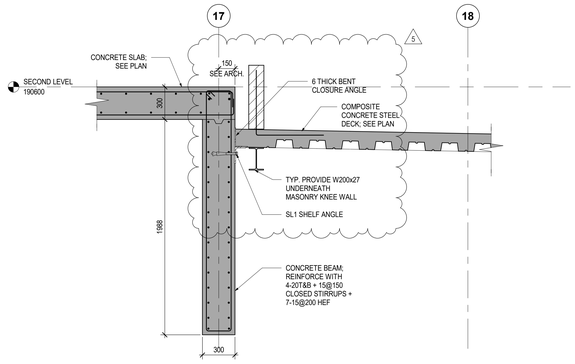

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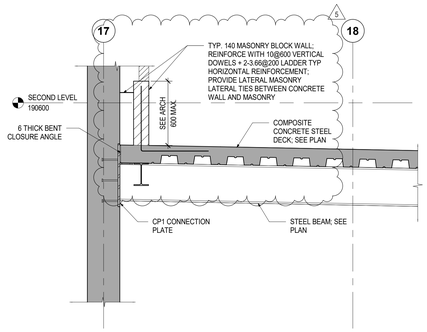

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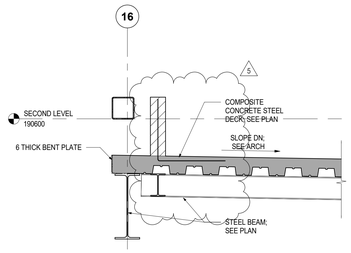

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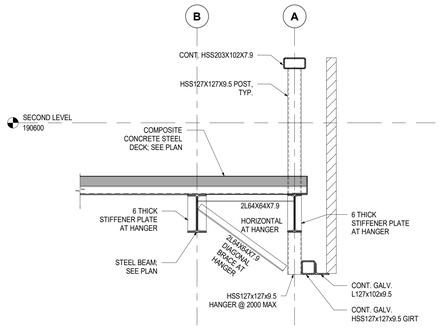

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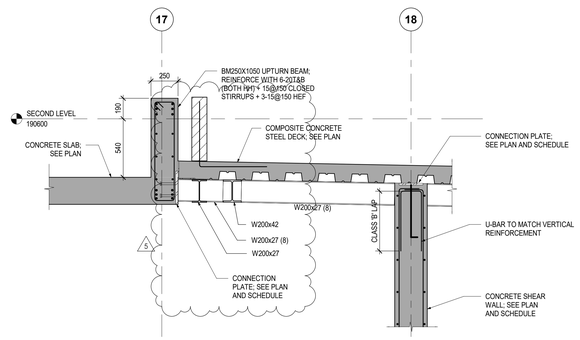

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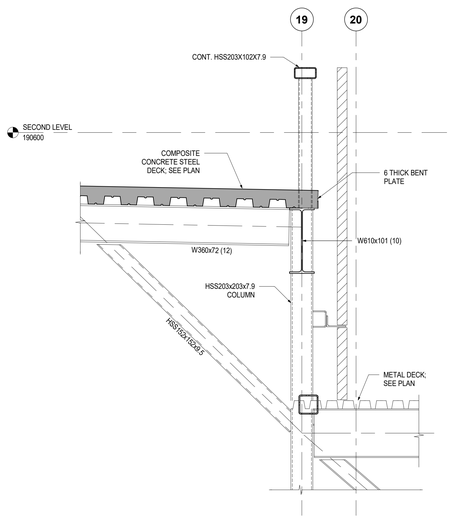

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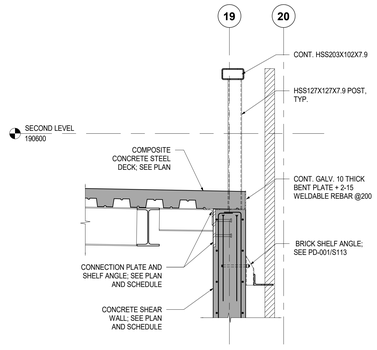

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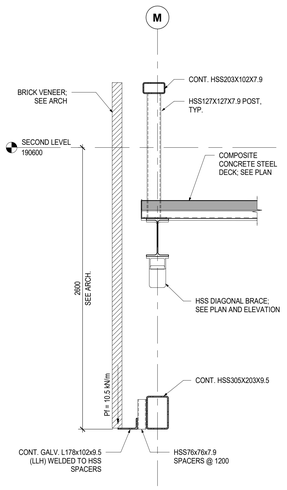

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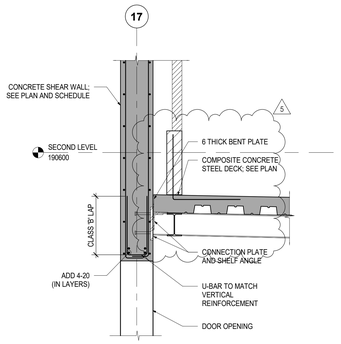

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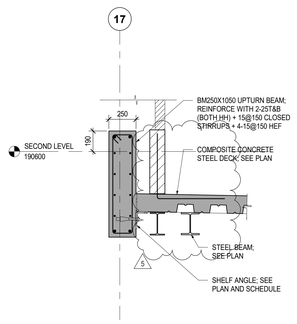

12 / S530 SECTION 1 : 20

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REGISTERED PROFESSIONAL ENGINEER — 2026-07-22 — E.J.CHOMOKHOS — 100072522 — PROVINCE OF ONTARIO

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THE STERLING HALL SCHOOL

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SECOND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S530
	Revision: 5

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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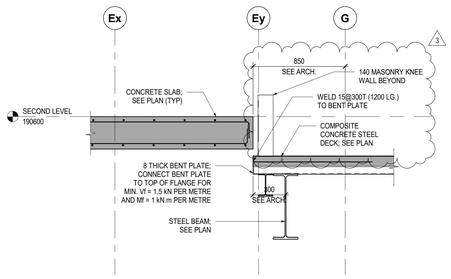

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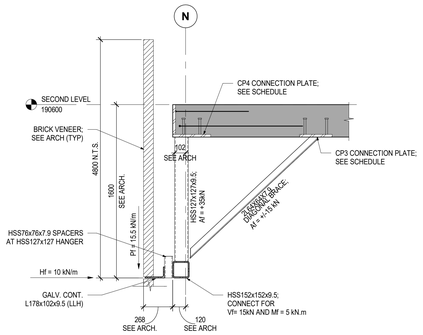

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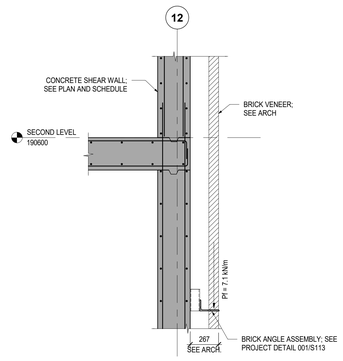

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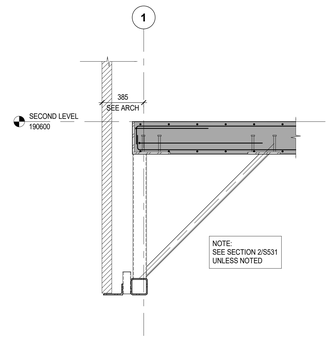

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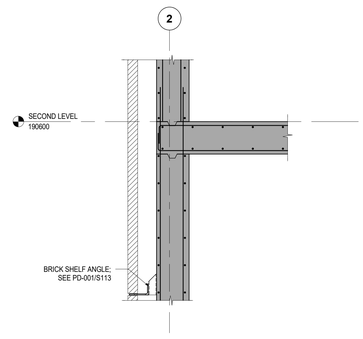

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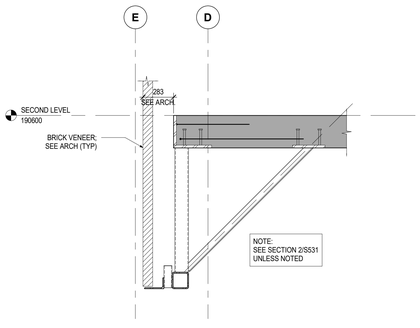
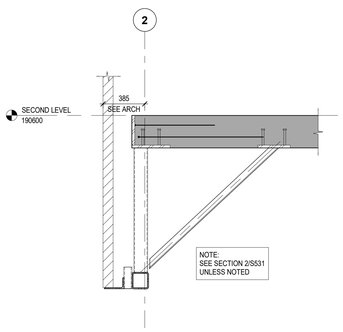
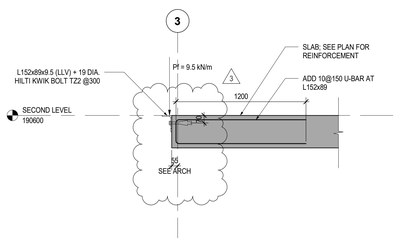


6 / S531 SECTION 1 : 20



7 / S531 SECTION 1 : 20



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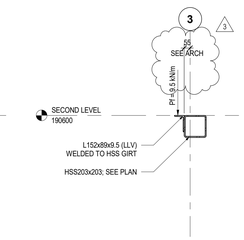
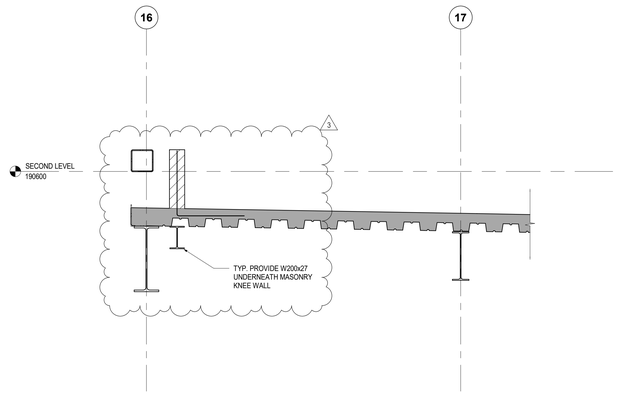


9 / S531 SECTION 1 : 20



10 / S531 SECTION 1 : 20

Key to Detail Location

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3	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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structural engineer	LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015
mechanical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
electrical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
landscape architect	Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

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SECOND FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S531
	Revision: 3

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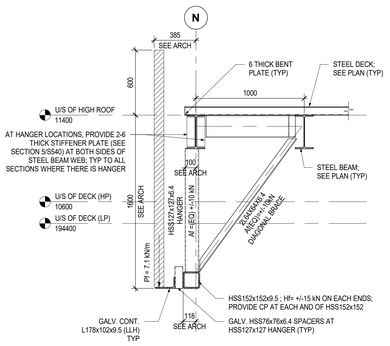

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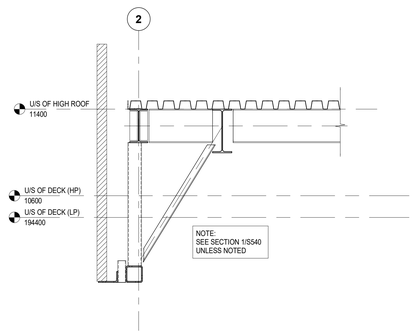

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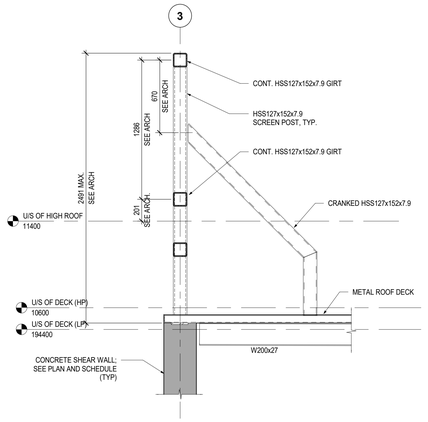

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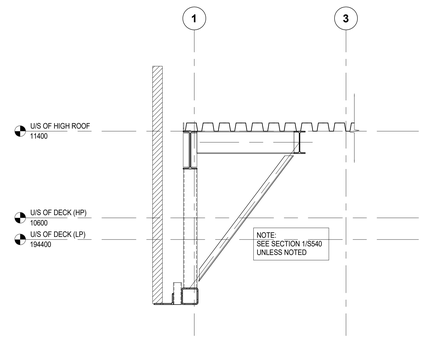

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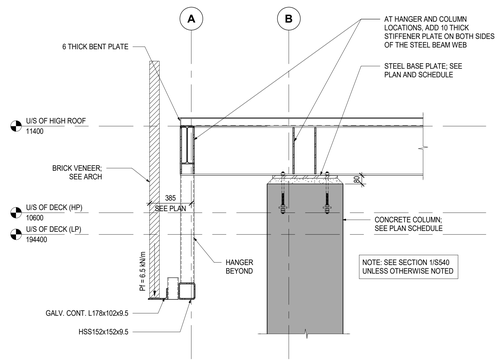

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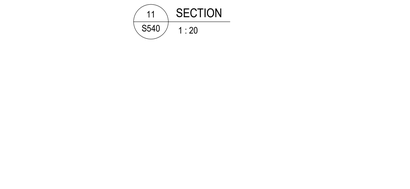

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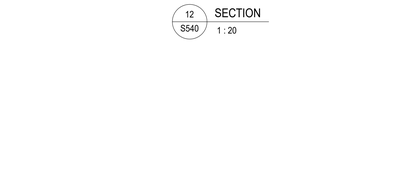

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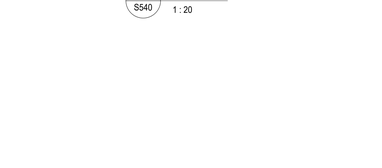
8 / S540 SECTION 1 : 20



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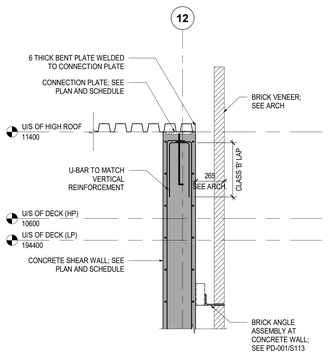

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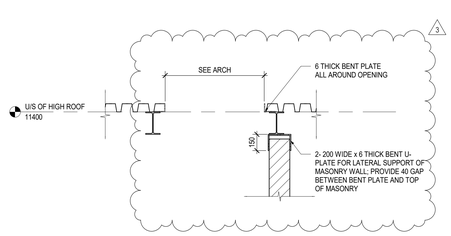

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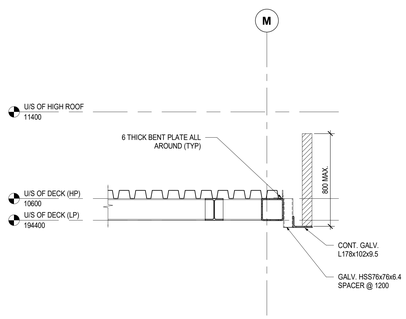

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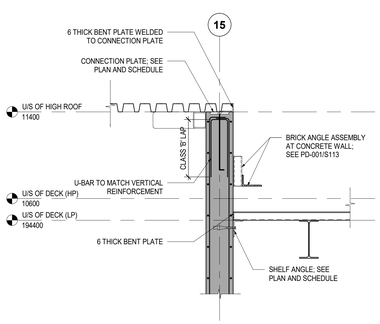

14 / S540 SECTION 1 : 20

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E.V.C. HOBSHURY
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ROOF FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S540
	Revision: 3

A0

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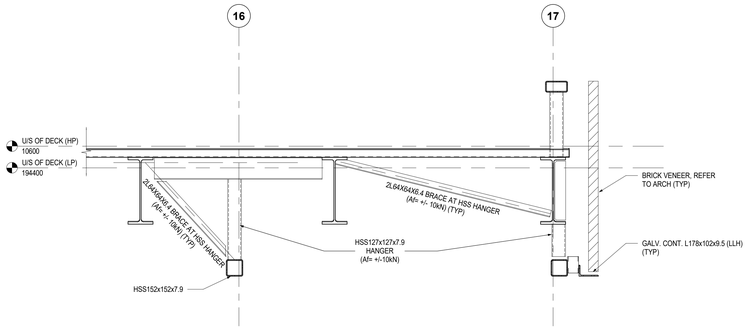

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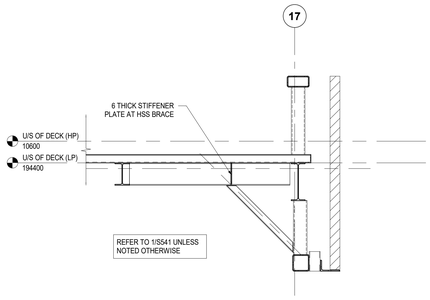

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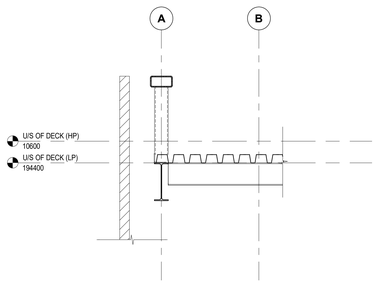

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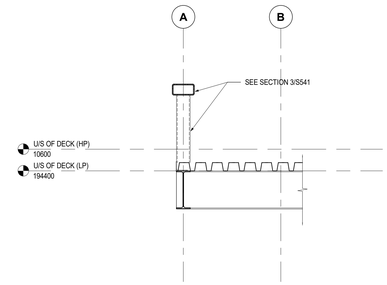

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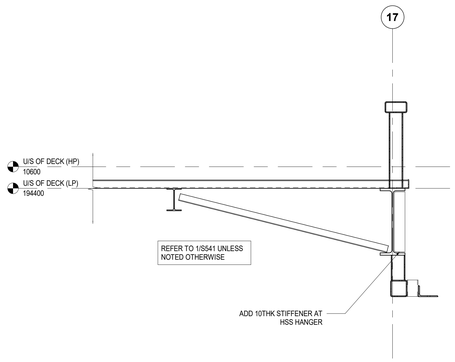

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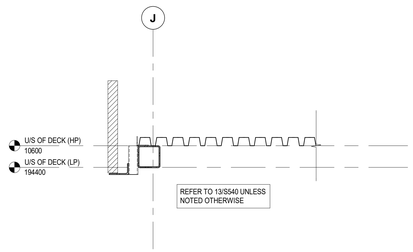

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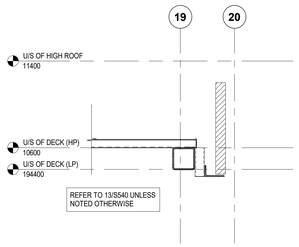

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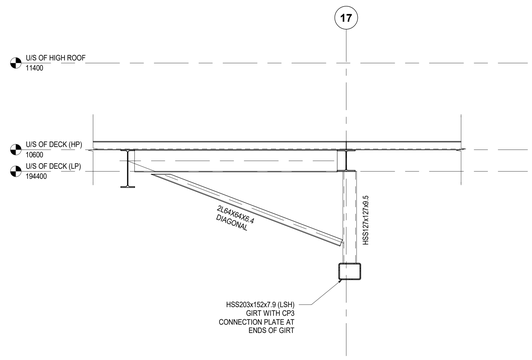

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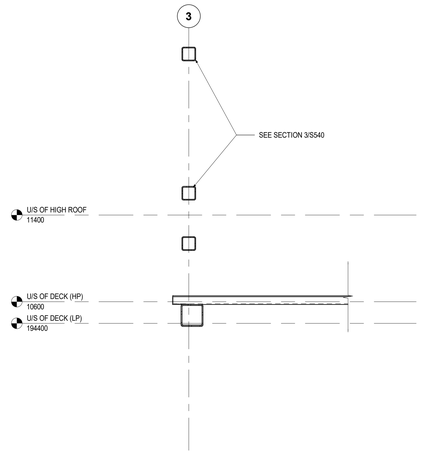

9 / S541 SECTION
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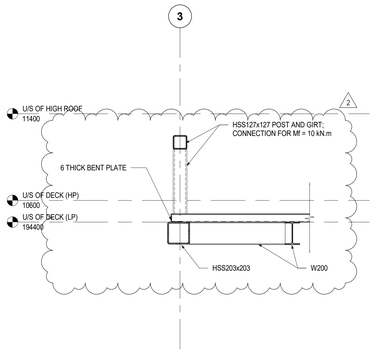

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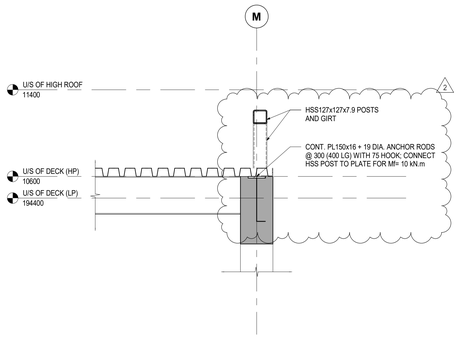

11 / S541 SECTION
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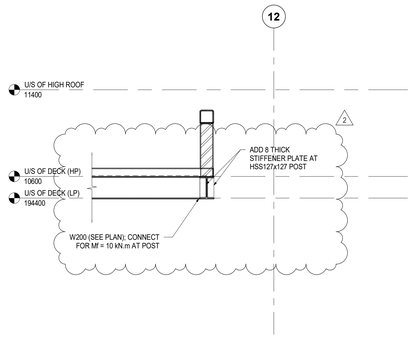

12 / S541 SECTION
1 : 20

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consultants

architect	CS&P Architects Inc. 2345 Yonge St., Suite 200 Toronto, ON, M4P 2E5 Phone.416.482.5002 Fax.416.482.5040
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structural engineer	LEA Consulting Ltd. 40 University Ave, Suite 503 Toronto, ON, M5J 1T1 Phone.905.470.0015
mechanical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
electrical engineer	Smith + Andersen 4211 Yonge Street Toronto, ON, M2P 2A9 Phone.416.487.8151
landscape architect	Quinn Design Associates Inc. 180 John Street Toronto, ON, M5T 1X5 Phone.416.962.8700

REGISTERED PROFESSIONAL ENGINEER
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E.U.C.HOWDHURY
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ROOF FLOOR SECTIONS

scale:	1 : 20
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S541
	Revision: 2

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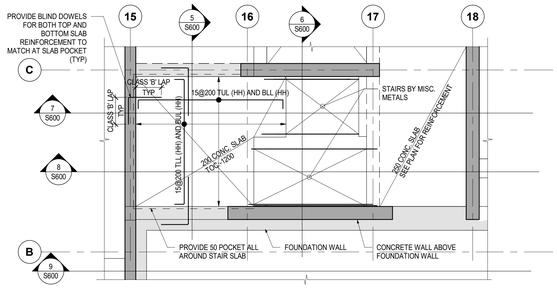

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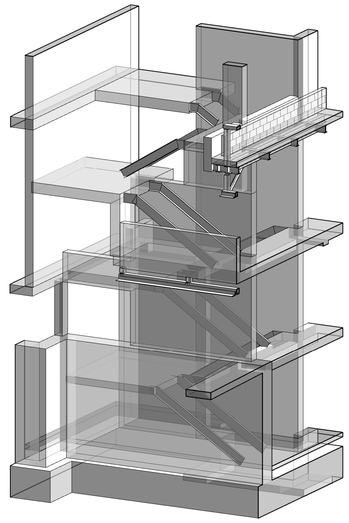
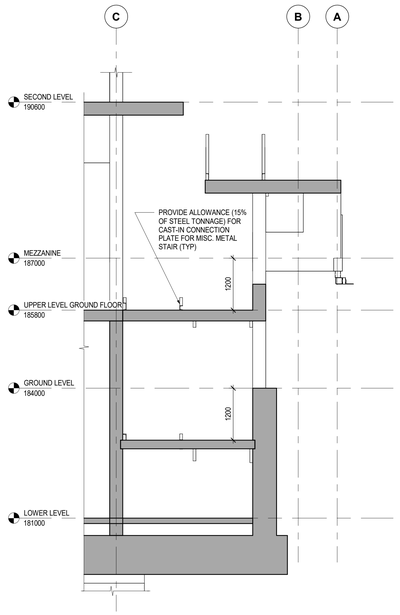
1 : 50



4 S600 STAIR 3D



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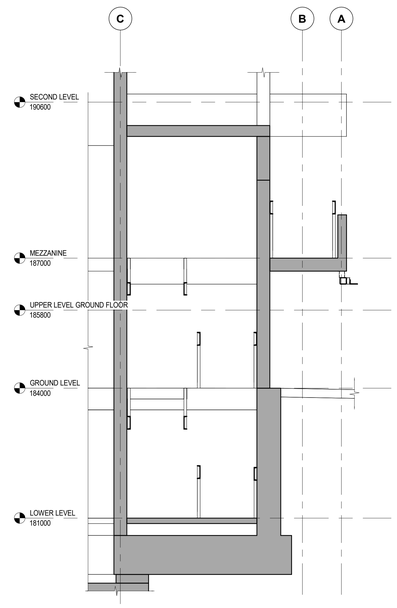

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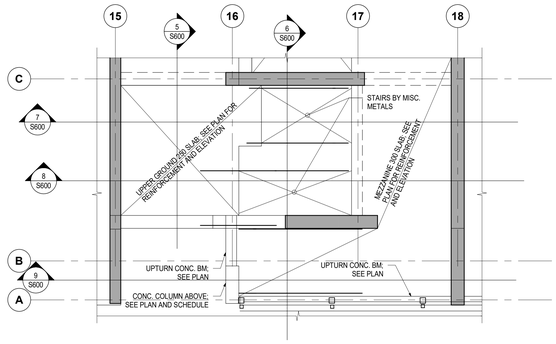

2 S600 UPPER GROUND TO MEZZANINE - STAIR PLAN
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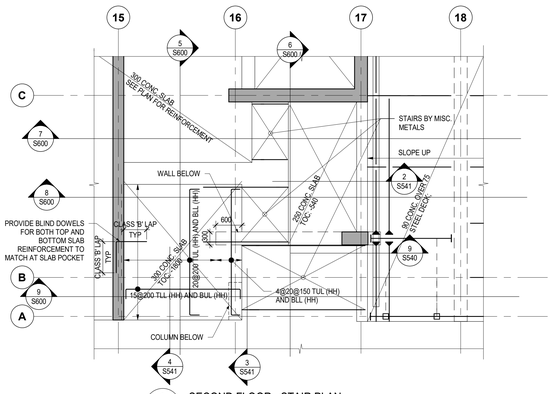

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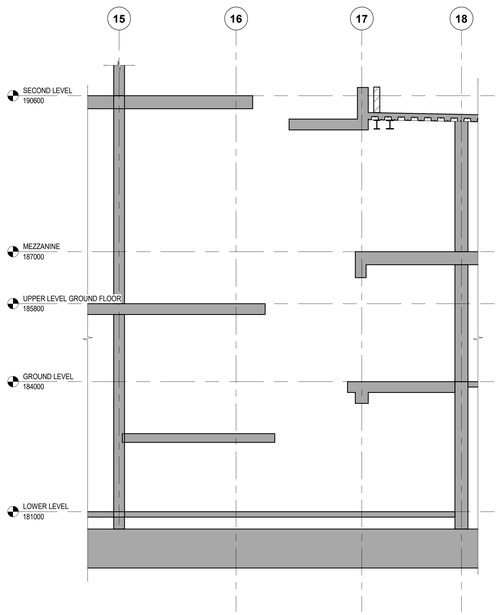

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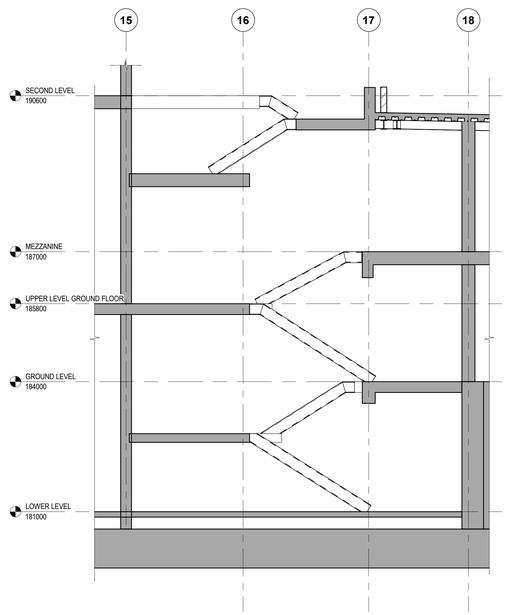

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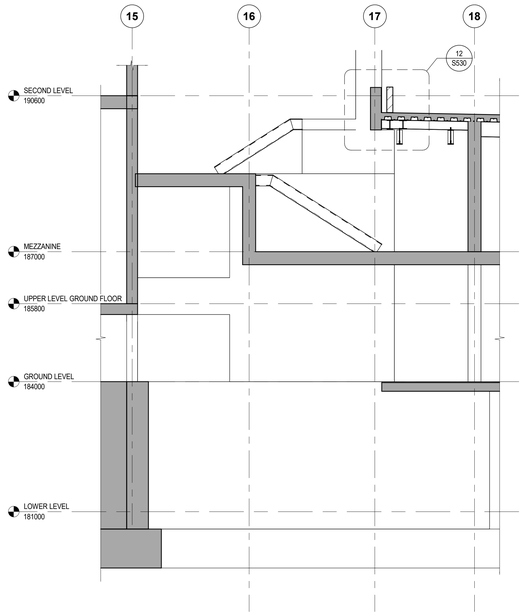

9 S600 SECTION
1 : 50

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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S600
	Revision 3

A0 FILENAME: Autodesk Docs://2102 Sterling Hall/22317_SHS-STR-R25.rvt

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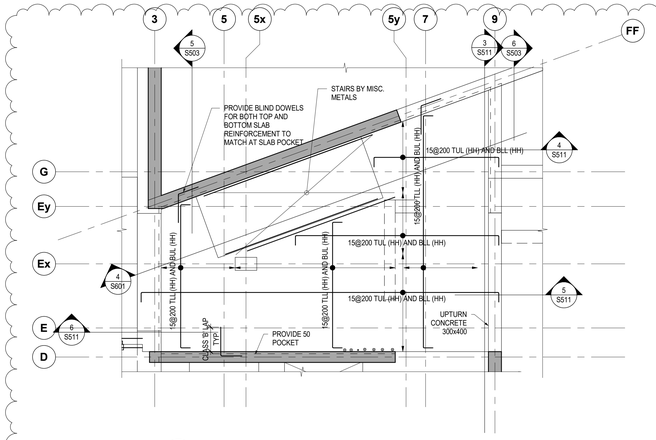

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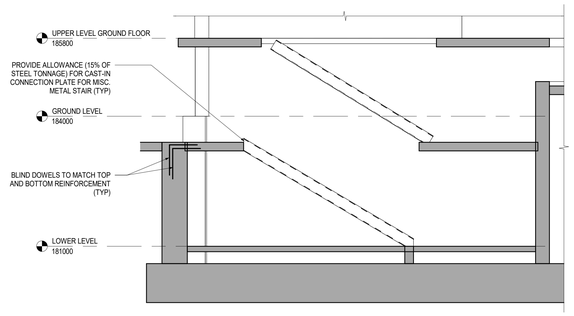

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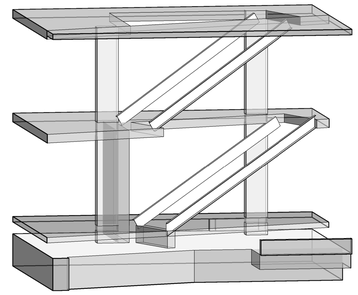

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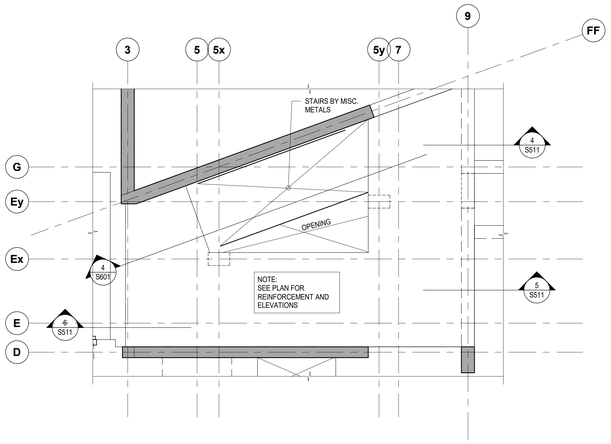

2 S601 UPPER GROUND FLOOR STAIR PLAN
1 : 50

Key to Detail Location

NO. Detail Number
NO. Drawing Number

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2	2026-07-08	ISSUED FOR TENDER
3	2026-07-22	ISSUED FOR TENDER ADDENDUM #1

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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S601
	Revision: 3

A0 FILENAME: Autodesk Docs://2102 Sterling Hall/22317_SHS-STR-R25.rvt

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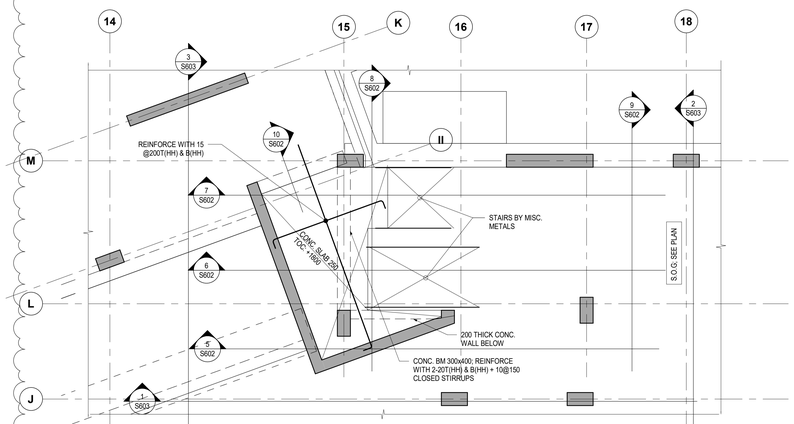

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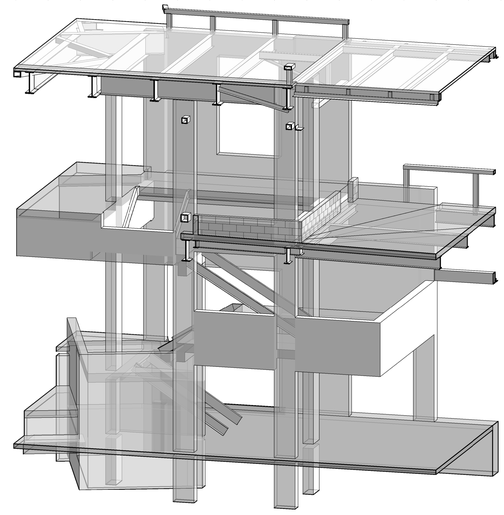

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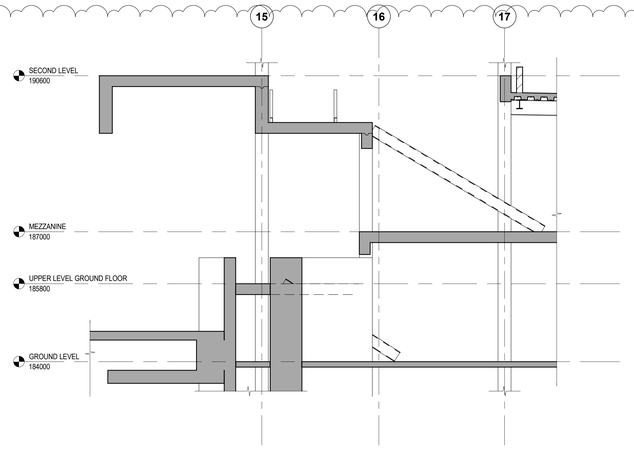

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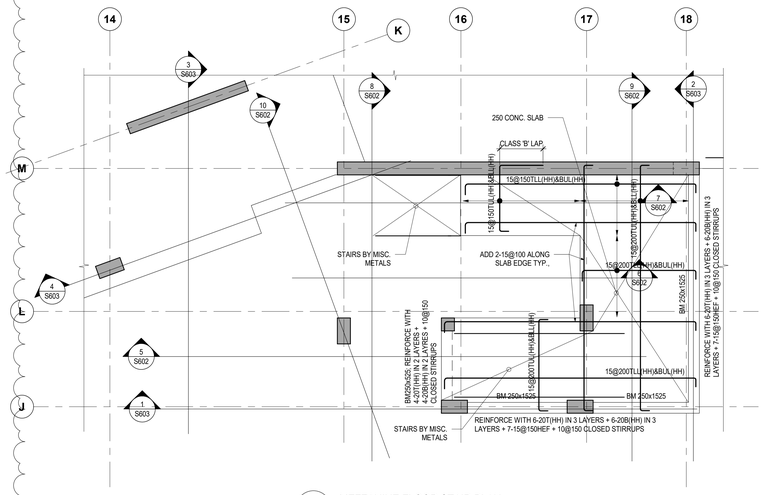

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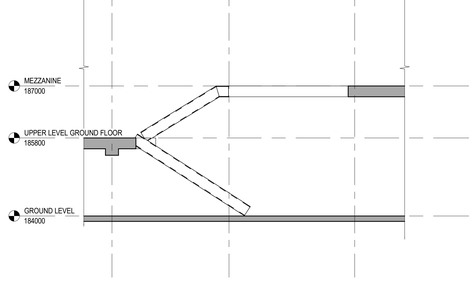

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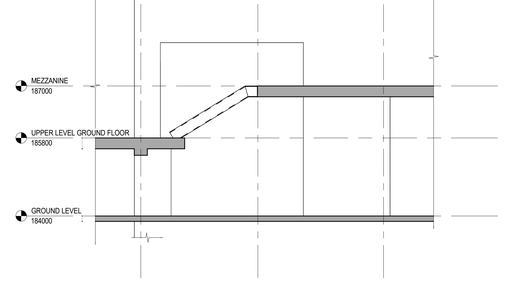

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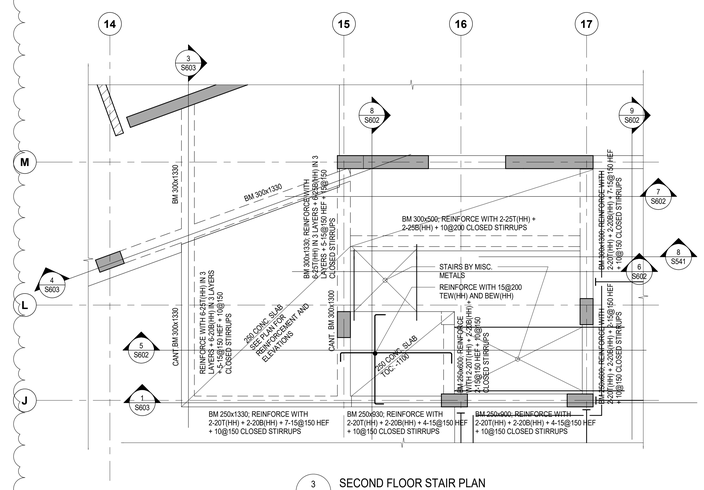

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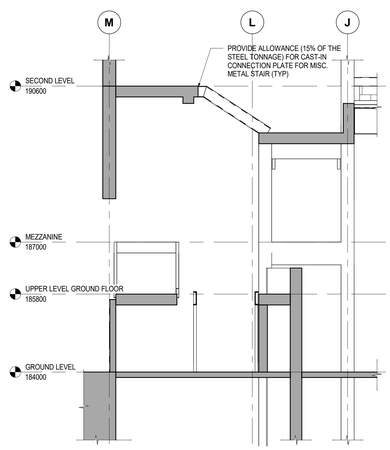

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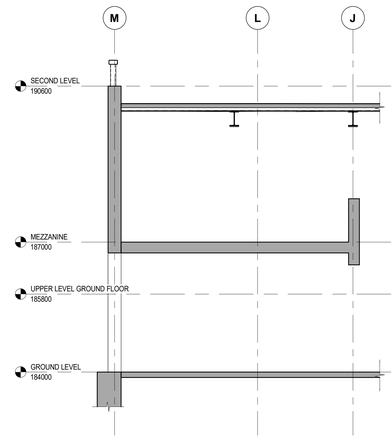

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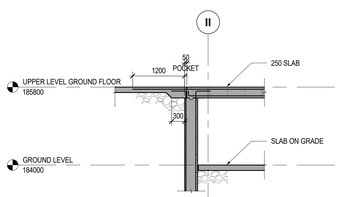

10 S602 SECTION
1 : 50

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STAIR PLANS AND SECTIONS

scale:	1 : 50
date:	2026-07-22
drawn:	H.R.
checked by:	E.C.
project number:	22317
drawing number:	S602
	Revision: 3

A0 FILENAME: Autodesk Docs://21032 Sterling Hall/22317_SHS-STR-R25.rvt

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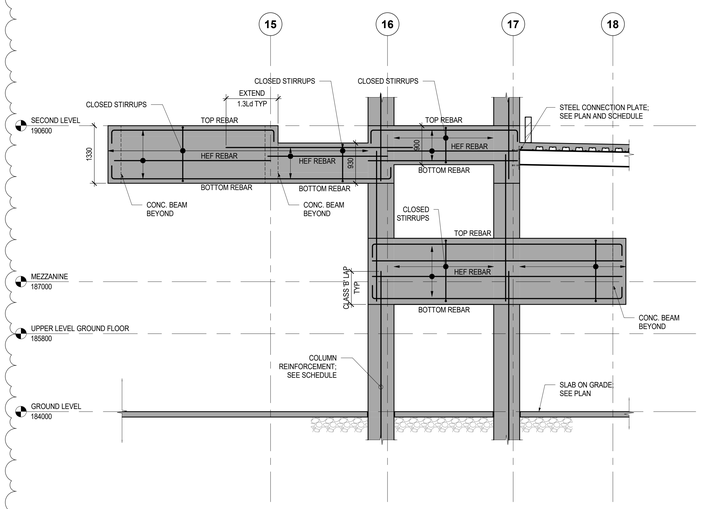

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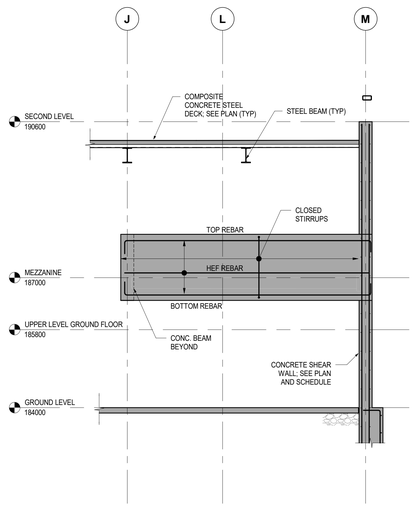

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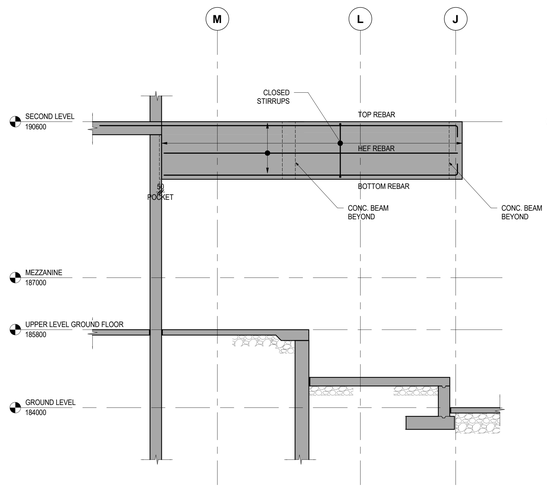

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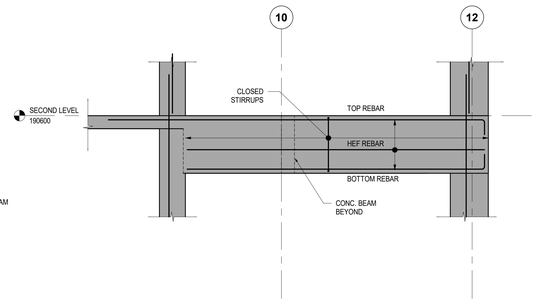

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